

CITY OF REDLANDS  
MASTER PLAN OF DRAINAGE (MPD)



**TECHNICAL APPENDIX A.3**

**Opal Basin Hydrology (AMC II)  
(100-year)**

\*\*\*\*\*  
RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
(Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)  
(c) Copyright 1983-2013 Advanced Engineering Software (aes)  
Ver. 20.0 Release Date: 06/01/2013 License ID 1264

Analysis prepared by:

\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* REDLANDS MPD - UPDATE \*  
\* REVISED RATIONAL METHOD HYDROLOGY - TO NODE 20151 \*  
\* BY TMULI SEPT 2013 \*  
\*\*\*\*\*

FILE NAME: LR0201.DAT  
TIME/DATE OF STUDY: 15:28 09/24/2013

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--\*TIME-OF-CONCENTRATION MODEL\*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00  
SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00  
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85  
\*USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL\*

SLOPE OF INTENSITY DURATION CURVE (LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000  
USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.2500

\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD\*

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF- CROWN TO	STREET-CROSSFALL:		CURB HEIGHT	GUTTER-GEOMETRIES:			MANNING FACTOR
	WIDTH CROSSFALL	IN- / OUT-	PARK- / SIDE/ WAY		WIDTH	LIP	HIKE	
====	====	====	====	====	====	====	====	====
1	18.0	12.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
2	20.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
3	22.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
4	15.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
5	18.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
6	15.0	10.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
7	16.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
8	16.0	10.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
9	17.0	10.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
10	30.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
11	24.0	15.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
12	24.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
13	32.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
14	39.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
15	36.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
16	12.5	5.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180

17	20.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
18	26.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
19	52.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.20 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)

\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*

\*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

UNIT-HYDROGRAPH MODEL SELECTIONS/PARAMETERS:

WATERSHED LAG = 0.80 \* Tc

S-GRAPH TYPE PERCENTAGE (DECIMAL)

VALLEY (DEVELOPED)	1.000
FOOTHILL	0.000
MOUNTAIN	0.000
VALLEY (UNDEVELOPED) / DESERT	0.000
DESERT (UNDEVELOPED)	0.000

PRECIPITATION DATA ENTERED ON SUBAREA BASIS.

SIERRA MADRE DEPTH-AREA FACTORS USED.

\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR UNIT HYDROGRAPH METHOD\*

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20100.00 TO NODE 20101.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 219.52  
ELEVATION DATA: UPSTREAM (FEET) = 2400.00 DOWNSTREAM (FEET) = 2385.00

Tc = K \* [(LENGTH\*\* 3.00) / (ELEVATION CHANGE)]\*\*0.20

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 6.474

\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.755

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
NATURAL FAIR COVER						
"OPEN BRUSH"	B	1.33	0.61	1.000	66	10.43
RESIDENTIAL						
"2 DWELLINGS/ACRE"	B	2.55	0.75	0.700	56	6.47
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.69						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.805						
SUBAREA RUNOFF (CFS) = 14.81						
TOTAL AREA (ACRES) = 3.92 PEAK FLOW RATE (CFS) = 14.81						

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 9.00

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20101.00 TO NODE 20102.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>(STREET TABLE SECTION # 5 USED)<<<<

UPSTREAM ELEVATION(FEET) = 2385.00 DOWNSTREAM ELEVATION(FEET) = 2340.00  
STREET LENGTH(FEET) = 138.73 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 22.82

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.32  
HALFSTREET FLOOD WIDTH(FEET) = 9.84  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 10.51  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 3.39  
STREET FLOW TRAVEL TIME(MIN.) = 0.22 Tc(MIN.) = 6.69  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.660

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER					
"OPEN BRUSH"	A	0.45	0.86	1.000	46
NATURAL FAIR COVER					
"OPEN BRUSH"	B	0.90	0.61	1.000	66
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	3.01	0.75	0.700	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.73  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.793  
SUBAREA AREA(ACRES) = 4.36 SUBAREA RUNOFF(CFS) = 16.02  
EFFECTIVE AREA(ACRES) = 8.28 AREA-AVERAGED Fm(INCH/HR) = 0.57  
AREA-AVERAGED Fp(INCH/HR) = 0.71 AREA-AVERAGED Ap = 0.80  
TOTAL AREA(ACRES) = 8.3 PEAK FLOW RATE(CFS) = 30.50

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 8.79

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.35 HALFSTREET FLOOD WIDTH(FEET) = 11.09  
FLOW VELOCITY(FEET/SEC.) = 11.32 DEPTH\*VELOCITY(FT\*FT/SEC.) = 3.94  
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20102.00 = 358.25 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20102.00 TO NODE 20103.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>(STREET TABLE SECTION # 5 USED)<<<<

UPSTREAM ELEVATION(FEET) = 2340.00 DOWNSTREAM ELEVATION(FEET) = 2320.00  
STREET LENGTH(FEET) = 287.27 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00

INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 42.06

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.47  
HALFSTREET FLOOD WIDTH(FEET) = 17.26  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.79  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 3.20  
STREET FLOW TRAVEL TIME(MIN.) = 0.70 Tc(MIN.) = 7.40  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.389

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER					
"OPEN BRUSH"	A	1.17	0.86	1.000	46
NATURAL FAIR COVER					
"OPEN BRUSH"	B	2.63	0.61	1.000	66
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	3.01	0.75	0.700	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.71  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.867  
SUBAREA AREA(ACRES) = 6.81 SUBAREA RUNOFF(CFS) = 23.12  
EFFECTIVE AREA(ACRES) = 15.09 AREA-AVERAGED Fm(INCH/HR) = 0.59  
AREA-AVERAGED Fp(INCH/HR) = 0.71 AREA-AVERAGED Ap = 0.83  
TOTAL AREA(ACRES) = 15.1 PEAK FLOW RATE(CFS) = 51.59

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 8.63

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.50 HALFSTREET FLOOD WIDTH(FEET) = 18.00  
FLOW VELOCITY(FEET/SEC.) = 7.23 DEPTH\*VELOCITY(FT\*FT/SEC.) = 3.60  
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20103.00 = 645.52 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20103.00 TO NODE 20104.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>(STREET TABLE SECTION # 5 USED)<<<<

UPSTREAM ELEVATION(FEET) = 2320.00 DOWNSTREAM ELEVATION(FEET) = 2310.00  
STREET LENGTH(FEET) = 249.70 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00

INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 97.01  
 \*\*\*STREET FLOWING FULL\*\*\*  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.64  
 HALFSTREET FLOOD WIDTH(FEET) = 24.79  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 7.45  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 4.74  
 STREET FLOW TRAVEL TIME(MIN.) = 0.56 Tc(MIN.) = 7.96  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.201  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER "OPEN BRUSH"	A	1.82	0.86	1.000	46
NATURAL FAIR COVER "OPEN BRUSH"	B	19.46	0.61	1.000	66
RESIDENTIAL "2 DWELLINGS/ACRE"	B	6.79	0.75	0.700	56
RESIDENTIAL "2 DWELLINGS/ACRE"	A	0.01	0.98	0.700	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.66  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.927  
 SUBAREA AREA(ACRES) = 28.08 SUBAREA RUNOFF(CFS) = 90.80  
 EFFECTIVE AREA(ACRES) = 43.17 AREA-AVERAGED Fm(INCH/HR) = 0.60  
 AREA-AVERAGED Fp(INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.89  
 TOTAL AREA(ACRES) = 43.2 PEAK FLOW RATE(CFS) = 139.84

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.80

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.71 HALFSTREET FLOOD WIDTH(FEET) = 28.51  
 FLOW VELOCITY(FEET/SEC.) = 8.23 DEPTH\*VELOCITY(FT\*FT/SEC.) = 5.85  
 \*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 249.7 FT WITH ELEVATION-DROP = 10.0 FT, IS 93.9 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20104.00  
 LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20104.00 = 895.22 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20104.00 TO NODE 20105.00 IS CODE = 62  
 -----  
 >>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<  
 =====  
 UPSTREAM ELEVATION(FEET) = 2310.00 DOWNSTREAM ELEVATION(FEET) = 2270.00  
 STREET LENGTH(FEET) = 747.57 CURB HEIGHT(INCHES) = 6.0  
 STREET HALFWIDTH(FEET) = 18.00  
 DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 220.14  
 \*\*\*STREET FLOWING FULL\*\*\*  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.78  
 HALFSTREET FLOOD WIDTH(FEET) = 32.17  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 10.27  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 8.05  
 STREET FLOW TRAVEL TIME(MIN.) = 1.21 Tc(MIN.) = 9.17  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.858  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER "OPEN BRUSH"	A	5.68	0.86	1.000	46
RESIDENTIAL "2 DWELLINGS/ACRE"	A	3.92	0.98	0.700	32
RESIDENTIAL "2 DWELLINGS/ACRE"	B	6.10	0.75	0.700	56
NATURAL FAIR COVER "OPEN BRUSH"	B	39.60	0.61	1.000	66

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.67  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.946  
 SUBAREA AREA(ACRES) = 55.30 SUBAREA RUNOFF(CFS) = 160.46  
 EFFECTIVE AREA(ACRES) = 98.47 AREA-AVERAGED Fm(INCH/HR) = 0.62  
 AREA-AVERAGED Fp(INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.92  
 TOTAL AREA(ACRES) = 98.5 PEAK FLOW RATE(CFS) = 286.98

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.72

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.86 HALFSTREET FLOOD WIDTH(FEET) = 35.77  
 FLOW VELOCITY(FEET/SEC.) = 10.90 DEPTH\*VELOCITY(FT\*FT/SEC.) = 9.32  
 \*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 747.6 FT WITH ELEVATION-DROP = 40.0 FT, IS 139.7 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20105.00  
 LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20105.00 = 1642.79 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20105.00 TO NODE 20106.00 IS CODE = 54  
 -----  
 >>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<  
 =====  
 ELEVATION DATA: UPSTREAM(FEET) = 2270.00 DOWNSTREAM(FEET) = 2230.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 1238.14 CHANNEL SLOPE = 0.0323  
 CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 2.000  
 MANNING'S FACTOR = 0.035 MAXIMUM DEPTH(FEET) = 3.30  
 CHANNEL FLOW THRU SUBAREA(CFS) = 286.98  
 FLOW VELOCITY(FEET/SEC.) = 10.51 FLOW DEPTH(FEET) = 2.65  
 TRAVEL TIME(MIN.) = 1.96 Tc(MIN.) = 11.13  
 LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20106.00 = 2880.93 FEET.  
 \*\*\*\*\*  
 FLOW PROCESS FROM NODE 20106.00 TO NODE 20106.00 IS CODE = 81  
 -----  
 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<  
 =====

MAINLINE Tc(MIN.) = 11.13  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.434  
 SUBAREA LOSS RATE DATA(AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER "OPEN BRUSH"	A	2.42	0.86	1.000	46
RESIDENTIAL "2 DWELLINGS/ACRE"	A	7.44	0.98	0.700	32
RESIDENTIAL "2 DWELLINGS/ACRE"	B	21.25	0.75	0.700	56
NATURAL FAIR COVER "OPEN BRUSH"	B	127.72	0.61	1.000	66

  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.64  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.946  
 SUBAREA AREA(ACRES) = 158.83 SUBAREA RUNOFF(CFS) = 403.88  
 EFFECTIVE AREA(ACRES) = 257.30 AREA-AVERAGED Fm(INCH/HR) = 0.61  
 AREA-AVERAGED Fp(INCH/HR) = 0.65 AREA-AVERAGED Ap = 0.94  
 TOTAL AREA(ACRES) = 257.3 PEAK FLOW RATE(CFS) = 653.29  
  
 SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.52

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20108.00 TO NODE 20107.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<  
 =====  
 ELEVATION DATA: UPSTREAM(FEET) = 2230.00 DOWNSTREAM(FEET) = 2170.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 1330.76 CHANNEL SLOPE = 0.0451  
 CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 2.000  
 MANNING'S FACTOR = 0.035 MAXIMUM DEPTH(FEET) = 10.00  
 CHANNEL FLOW THRU SUBAREA(CFS) = 653.29  
 FLOW VELOCITY(FEET/SEC.) = 11.75 FLOW DEPTH(FEET) = 1.67  
 TRAVEL TIME(MIN.) = 1.89 Tc(MIN.) = 13.02  
 LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20107.00 = 4211.69 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20107.00 TO NODE 20107.00 IS CODE = 81  
 -----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<  
 =====  
 MAINLINE Tc(MIN.) = 13.02  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.126  
 SUBAREA LOSS RATE DATA(AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER "OPEN BRUSH"	A	2.55	0.86	1.000	46
RESIDENTIAL "2 DWELLINGS/ACRE"	A	12.67	0.98	0.700	32
RESIDENTIAL "2 DWELLINGS/ACRE"	B	10.30	0.75	0.700	56
NATURAL FAIR COVER "OPEN BRUSH"	B	66.90	0.61	1.000	66

  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.67  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.925

SUBAREA AREA(ACRES) = 92.42 SUBAREA RUNOFF(CFS) = 208.45  
 EFFECTIVE AREA(ACRES) = 349.72 AREA-AVERAGED Fm(INCH/HR) = 0.61  
 AREA-AVERAGED Fp(INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.93  
 TOTAL AREA(ACRES) = 349.7 PEAK FLOW RATE(CFS) = 790.41

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20107.00 TO NODE 20108.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<  
 =====  
 ELEVATION DATA: UPSTREAM(FEET) = 2170.00 DOWNSTREAM(FEET) = 2095.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 1995.70 CHANNEL SLOPE = 0.0376  
 CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 2.000  
 MANNING'S FACTOR = 0.035 MAXIMUM DEPTH(FEET) = 10.00  
 CHANNEL FLOW THRU SUBAREA(CFS) = 790.41  
 FLOW VELOCITY(FEET/SEC.) = 11.81 FLOW DEPTH(FEET) = 1.97  
 TRAVEL TIME(MIN.) = 2.82 Tc(MIN.) = 15.84  
 LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20108.00 = 6207.39 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20108.00 TO NODE 20108.00 IS CODE = 81  
 -----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<  
 =====  
 MAINLINE Tc(MIN.) = 15.84  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.780  
 SUBAREA LOSS RATE DATA(AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER "OPEN BRUSH"	A	3.92	0.86	1.000	46
RESIDENTIAL "2 DWELLINGS/ACRE"	A	0.86	0.98	0.700	32
RESIDENTIAL "3-4 DWELLINGS/ACRE"	A	16.85	0.98	0.600	32
MOBILE HOME PARK RESIDENTIAL	B	25.39	0.75	0.250	56
"3-4 DWELLINGS/ACRE"	B	10.75	0.75	0.600	56
NATURAL FAIR COVER "OPEN BRUSH"	B	87.64	0.61	1.000	66

  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.67  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.791  
 SUBAREA AREA(ACRES) = 145.41 SUBAREA RUNOFF(CFS) = 294.30  
 EFFECTIVE AREA(ACRES) = 495.13 AREA-AVERAGED Fm(INCH/HR) = 0.59  
 AREA-AVERAGED Fp(INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.89  
 TOTAL AREA(ACRES) = 495.1 PEAK FLOW RATE(CFS) = 975.67

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20108.00 TO NODE 20109.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 2095.00 DOWNSTREAM(FEET) = 2020.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 2023.91 CHANNEL SLOPE = 0.0371
CHANNEL BASE(FEET) = 40.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.035 MAXIMUM DEPTH(FEET) = 10.00
CHANNEL FLOW THRU SUBAREA(CFS) = 975.67
FLOW VELOCITY(FEET/SEC.) = 11.71 FLOW DEPTH(FEET) = 1.90
TRAVEL TIME(MIN.) = 2.88 Tc(MIN.) = 18.72
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20109.00 = 8231.30 FEET.

\*\*\*\*\*
FLOW PROCESS FROM NODE 20109.00 TO NODE 20109.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

MAINLINE Tc(MIN.) = 18.72
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.514
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
NATURAL FAIR COVER
"OPEN BRUSH" A 2.81 0.86 1.000 46
RESIDENTIAL
"3-4 DWELLINGS/ACRE" A 27.06 0.98 0.600 32
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B 26.94 0.75 0.600 56
RESIDENTIAL
"2 DWELLINGS/ACRE" B 35.77 0.75 0.700 56
NATURAL FAIR COVER
"OPEN BRUSH" B 102.40 0.61 1.000 66
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.69
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.834
SUBAREA AREA(ACRES) = 194.98 SUBAREA RUNOFF(CFS) = 340.50
EFFECTIVE AREA(ACRES) = 690.11 AREA-AVERAGED Fm(INCH/HR) = 0.59
AREA-AVERAGED Fp(INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.88
TOTAL AREA(ACRES) = 690.1 PEAK FLOW RATE(CFS) = 1197.96

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*
FLOW PROCESS FROM NODE 20109.00 TO NODE 20109.00 IS CODE = 71

>>>>PEAK FLOW RATE ESTIMATOR CHANGED TO UNIT-HYDROGRAPH METHOD<<<<
>>>>USING TIME-OF-CONCENTRATION OF LONGEST FLOWPATH<<<<

UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.46;30M= 0.95;1H= 1.25;3H= 2.25;6H= 3.25;24H= 7.56
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.31; LAG(HR) = 0.25; Fm(INCH/HR) = 0.59; Ybar = 0.51
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;
3HR = 1.00; 6HR = 1.00; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 690.1
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20109.00 = 8231.30 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3,n=.0333; Lca/L=0.4,n=.0299; Lca/L=0.5,n=.0274;Lca/L=0.6,n=.0256
TIME OF PEAK FLOW(HR) = 16.33 RUNOFF VOLUME(AF) = 224.38
UNIT-HYDROGRAPH METHOD PEAK FLOW RATE(CFS) = 1236.59
TOTAL PEAK FLOW RATE(CFS) = 1236.59 (SOURCE FLOW INCLUDED)
RATIONAL METHOD PEAK FLOW RATE(CFS) = 1197.96
(UPSTREAM NODE PEAK FLOW RATE(CFS) = 1197.96)
PEAK FLOW RATE(CFS) USED = 1236.59

\*\*\*\*\*
FLOW PROCESS FROM NODE 20109.00 TO NODE 20110.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 2020.00 DOWNSTREAM(FEET) = 1960.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1927.24 CHANNEL SLOPE = 0.0311
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.035 MAXIMUM DEPTH(FEET) = 5.00
CHANNEL FLOW THRU SUBAREA(CFS) = 1236.59
FLOW VELOCITY(FEET/SEC.) = 14.88 FLOW DEPTH(FEET) = 4.41
TRAVEL TIME(MIN.) = 2.16 Tc(MIN.) = 20.88
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20110.00 = 10158.54 FEET.

\*\*\*\*\*
FLOW PROCESS FROM NODE 20110.00 TO NODE 20110.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

MAINLINE Tc(MIN.) = 20.88
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.355
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
NATURAL FAIR COVER
"OPEN BRUSH" A 5.83 0.86 1.000 46
RESIDENTIAL
"3-4 DWELLINGS/ACRE" A 33.80 0.98 0.600 32
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B 25.19 0.75 0.600 56
RESIDENTIAL
"2 DWELLINGS/ACRE" B 9.84 0.75 0.700 56
NATURAL FAIR COVER
"OPEN BRUSH" B 45.99 0.61 1.000 66
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.780
SUBAREA AREA(ACRES) = 120.65

UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.46;30M= 0.95;1H= 1.25;3H= 2.25;6H= 3.25;24H= 7.55
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.35; LAG(HR) = 0.28; Fm(INCH/HR) = 0.58; Ybar = 0.51
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;
3HR = 0.99; 6HR = 1.00; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 810.8
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20110.00 = 10158.54 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0313; Lca/L=0.4,n=.0280; Lca/L=0.5,n=.0258;Lca/L=0.6,n=.0240

TIME OF PEAK FLOW(HR) = 16.33 RUNOFF VOLUME(AF) = 262.28  
UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 1349.99  
TOTAL AREA(ACRES) = 810.8 PEAK FLOW RATE(CFS) = 1349.99

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20110.00 TO NODE 20111.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<  
=====

ELEVATION DATA: UPSTREAM(FEET) =	1960.00	DOWNSTREAM(FEET) =	1920.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	928.33	CHANNEL SLOPE =	0.0431
CHANNEL BASE(FEET) =	10.00	"Z" FACTOR =	2.000
MANNING'S FACTOR =	0.035	MAXIMUM DEPTH(FEET) =	5.00
CHANNEL FLOW THRU SUBAREA(CFS) =	1349.99		
FLOW VELOCITY(FEET/SEC.) =	17.14	FLOW DEPTH(FEET) =	4.25
TRAVEL TIME(MIN.) =	0.90	Tc(MIN.) =	21.78
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20111.00 =	11086.87	FEET.	

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20111.00 TO NODE 20111.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<  
=====

MAINLINE Tc(MIN.) = 21.78  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.296  
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER "OPEN BRUSH"	A	28.59	0.86	1.000	46
RESIDENTIAL "3-4 DWELLINGS/ACRE"	A	31.08	0.98	0.600	32
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	31.56	0.75	0.600	56
NATURAL FAIR COVER "OPEN BRUSH"	B	41.72	0.61	1.000	66
RESIDENTIAL ".4 DWELLING/ACRE"	B	5.26	0.75	0.900	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.76  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.815  
SUBAREA AREA(ACRES) = 138.21  
UNIT-HYDROGRAPH DATA:  
RAINFALL(INCH): 5M= 0.46;30M= 0.95;1H= 1.25;3H= 2.24;6H= 3.22;24H= 7.55  
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%  
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%  
Tc(HR) = 0.36; LAG(HR) = 0.29; Fm(INCH/HR) = 0.59; Ybar = 0.52  
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.  
DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;  
3HR = 0.99; 6HR = 1.00; 24HR= 1.00  
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 949.0  
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20111.00 = 11086.87 FEET.  
EQUIVALENT BASIN FACTOR APPROXIMATIONS:  
Lca/L=0.3,n=.0305; Lca/L=0.4,n=.0274; Lca/L=0.5,n=.0251;Lca/L=0.6,n=.0235  
TIME OF PEAK FLOW(HR) = 16.33 RUNOFF VOLUME(AF) = 301.99

UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 1499.62  
TOTAL AREA(ACRES) = 949.0 PEAK FLOW RATE(CFS) = 1499.62

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.16; 6HR = 3.06; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20111.00 TO NODE 20112.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<  
=====

ELEVATION DATA: UPSTREAM(FEET) =	1920.00	DOWNSTREAM(FEET) =	1870.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	1664.97	CHANNEL SLOPE =	0.0300
CHANNEL BASE(FEET) =	10.00	"Z" FACTOR =	2.000
MANNING'S FACTOR =	0.035	MAXIMUM DEPTH(FEET) =	5.00
CHANNEL FLOW THRU SUBAREA(CFS) =	1499.62		
FLOW VELOCITY(FEET/SEC.) =	15.44	FLOW DEPTH(FEET) =	4.90
TRAVEL TIME(MIN.) =	1.80	Tc(MIN.) =	23.58
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20112.00 =	12751.84	FEET.	

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20112.00 TO NODE 20112.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<  
=====

MAINLINE Tc(MIN.) = 23.58  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.189  
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "3-4 DWELLINGS/ACRE"	A	8.51	0.98	0.600	32
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	0.54	0.75	0.600	56
RESIDENTIAL ".4 DWELLING/ACRE"	A	3.29	0.98	0.900	32
RESIDENTIAL ".4 DWELLING/ACRE"	B	75.85	0.75	0.900	56
NATURAL FAIR COVER "OPEN BRUSH"	B	7.12	0.61	1.000	66

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.76  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.879  
SUBAREA AREA(ACRES) = 95.31  
UNIT-HYDROGRAPH DATA:  
RAINFALL(INCH): 5M= 0.46;30M= 0.95;1H= 1.25;3H= 2.22;6H= 3.18;24H= 7.54  
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%  
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%  
Tc(HR) = 0.39; LAG(HR) = 0.31; Fm(INCH/HR) = 0.60; Ybar = 0.53  
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.  
DEPTH-AREA FACTORS: 5M = 0.95; 30M = 0.95; 1HR = 0.95;  
3HR = 0.99; 6HR = 1.00; 24HR= 1.00  
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 1044.3  
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20112.00 = 12751.84 FEET.  
EQUIVALENT BASIN FACTOR APPROXIMATIONS:  
Lca/L=0.3,n=.0295; Lca/L=0.4,n=.0264; Lca/L=0.5,n=.0243;Lca/L=0.6,n=.0227  
TIME OF PEAK FLOW(HR) = 16.42 RUNOFF VOLUME(AF) = 327.56  
UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 1525.78

TOTAL AREA (ACRES) = 1044.3 PEAK FLOW RATE (CFS) = 1525.78

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20112.00 TO NODE 20150.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1870.00 DOWNSTREAM (FEET) = 1850.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 907.32 CHANNEL SLOPE = 0.0220  
CHANNEL BASE (FEET) = 10.00 "Z" FACTOR = 2.000  
MANNING'S FACTOR = 0.035 MAXIMUM DEPTH (FEET) = 10.00  
CHANNEL FLOW THRU SUBAREA (CFS) = 1525.78  
FLOW VELOCITY (FEET/SEC.) = 13.86 FLOW DEPTH (FEET) = 5.33  
TRAVEL TIME (MIN.) = 1.09 Tc (MIN.) = 24.67  
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20150.00 = 13659.16 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20150.00 TO NODE 20150.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc (MIN.) = 24.67  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.131  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	4.19	0.75	0.600	56
RESIDENTIAL ".4 DWELLING/ACRE"	B	3.83	0.75	0.900	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.743  
SUBAREA AREA (ACRES) = 8.02  
UNIT-HYDROGRAPH DATA:  
RAINFALL (INCH): 5M = 0.46; 30M = 0.95; 1H = 1.25; 3H = 2.22; 6H = 3.18; 24H = 7.54  
S-GRAPH: VALLEY (DEV.) = 100.0%; VALLEY (UNDEV.) / DESERT = 0.0%  
MOUNTAIN = 0.0%; FOOTHILL = 0.0%; DESERT (UNDEV.) = 0.0%  
Tc (HR) = 0.41; LAG (HR) = 0.33; Fm (INCH/HR) = 0.60; Ybar = 0.53  
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.  
DEPTH-AREA FACTORS: 5M = 0.95; 30M = 0.95; 1HR = 0.95;  
3HR = 0.99; 6HR = 1.00; 24HR = 1.00  
UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 1052.3  
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20150.00 = 13659.16 FEET.  
EQUIVALENT BASIN FACTOR APPROXIMATIONS:  
Lca/L = 0.3, n = .0291; Lca/L = 0.4, n = .0261; Lca/L = 0.5, n = .0240; Lca/L = 0.6, n = .0224  
TIME OF PEAK FLOW (HR) = 16.42 RUNOFF VOLUME (AF) = 330.18  
UNIT-HYDROGRAPH PEAK FLOW RATE (CFS) = 1524.49  
TOTAL AREA (ACRES) = 1052.3 PEAK FLOW RATE (CFS) = 1525.78  
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*

FLOW PROCESS FROM NODE 20150.00 TO NODE 20150.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20120.00 TO NODE 20121.00 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 591.56  
ELEVATION DATA: UPSTREAM (FEET) = 3148.00 DOWNSTREAM (FEET) = 2920.00

Tc = K \* [(LENGTH\*\* 3.00) / (ELEVATION CHANGE)]\*\*0.20  
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.975  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.464  
SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
NATURAL FAIR COVER "OPEN BRUSH"	B	5.75	0.61	1.000	66	10.98

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.61  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000  
SUBAREA RUNOFF (CFS) = 14.75  
TOTAL AREA (ACRES) = 5.75 PEAK FLOW RATE (CFS) = 14.75

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20121.00 TO NODE 20122.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 2920.00 DOWNSTREAM (FEET) = 2860.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 401.18 CHANNEL SLOPE = 0.1496  
CHANNEL BASE (FEET) = 0.00 "Z" FACTOR = 2.500  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH (FEET) = 1.00  
CHANNEL FLOW THRU SUBAREA (CFS) = 14.75  
FLOW VELOCITY (FEET/SEC.) = 7.20 FLOW DEPTH (FEET) = 0.91  
TRAVEL TIME (MIN.) = 0.93 Tc (MIN.) = 11.90  
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20122.00 = 992.74 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20122.00 TO NODE 20122.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc (MIN.) = 11.90  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.299  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER "OPEN BRUSH"	B	6.02	0.61	1.000	66

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.61



SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000  
SUBAREA AREA (ACRES) = 6.02 SUBAREA RUNOFF (CFS) = 14.55  
EFFECTIVE AREA (ACRES) = 11.77 AREA-AVERAGED Fm (INCH/HR) = 0.61  
AREA-AVERAGED Fp (INCH/HR) = 0.61 AREA-AVERAGED Ap = 1.00  
TOTAL AREA (ACRES) = 11.8 PEAK FLOW RATE (CFS) = 28.44

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20122.00 TO NODE 20123.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) =	2860.00	DOWNSTREAM (FEET) =	2800.00
CHANNEL LENGTH THRU SUBAREA (FEET) =	404.41	CHANNEL SLOPE =	0.1484
CHANNEL BASE (FEET) =	0.00	"Z" FACTOR =	2.500
MANNING'S FACTOR =	0.045	MAXIMUM DEPTH (FEET) =	2.00
CHANNEL FLOW THRU SUBAREA (CFS) =	28.44		
FLOW VELOCITY (FEET/SEC.) =	8.42	FLOW DEPTH (FEET) =	1.16
TRAVEL TIME (MIN.) =	0.80	Tc (MIN.) =	12.70
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20123.00 =	1397.15	FEET.	

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20123.00 TO NODE 20123.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) =	12.70				
* 100 YEAR RAINFALL INTENSITY (INCH/HR) =	3.173				
SUBAREA LOSS RATE DATA (AMC II):					
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
NATURAL FAIR COVER					
"OPEN BRUSH"	B	5.11	0.61	1.000	66
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) =	0.61				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	1.000				
SUBAREA AREA (ACRES) =	5.11	SUBAREA RUNOFF (CFS) =	11.77		
EFFECTIVE AREA (ACRES) =	16.88	AREA-AVERAGED Fm (INCH/HR) =	0.61		
AREA-AVERAGED Fp (INCH/HR) =	0.61	AREA-AVERAGED Ap =	1.00		
TOTAL AREA (ACRES) =	16.9	PEAK FLOW RATE (CFS) =	38.87		

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20123.00 TO NODE 20124.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) =	2800.00	DOWNSTREAM (FEET) =	2720.00
CHANNEL LENGTH THRU SUBAREA (FEET) =	734.74	CHANNEL SLOPE =	0.1089
CHANNEL BASE (FEET) =	0.00	"Z" FACTOR =	2.500
MANNING'S FACTOR =	0.045	MAXIMUM DEPTH (FEET) =	2.00
CHANNEL FLOW THRU SUBAREA (CFS) =	38.87		
FLOW VELOCITY (FEET/SEC.) =	8.12	FLOW DEPTH (FEET) =	1.38

TRAVEL TIME (MIN.) = 1.51 Tc (MIN.) = 14.21  
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20124.00 = 2131.89 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20124.00 TO NODE 20124.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) =	14.21				
* 100 YEAR RAINFALL INTENSITY (INCH/HR) =	2.966				
SUBAREA LOSS RATE DATA (AMC II):					
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
NATURAL FAIR COVER					
"OPEN BRUSH"	B	33.25	0.61	1.000	66
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) =	0.61				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	1.000				
SUBAREA AREA (ACRES) =	33.25	SUBAREA RUNOFF (CFS) =	70.39		
EFFECTIVE AREA (ACRES) =	50.13	AREA-AVERAGED Fm (INCH/HR) =	0.61		
AREA-AVERAGED Fp (INCH/HR) =	0.61	AREA-AVERAGED Ap =	1.00		
TOTAL AREA (ACRES) =	50.1	PEAK FLOW RATE (CFS) =	106.12		

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20124.00 TO NODE 20125.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) =	2720.00	DOWNSTREAM (FEET) =	2620.00
CHANNEL LENGTH THRU SUBAREA (FEET) =	932.28	CHANNEL SLOPE =	0.1073
CHANNEL BASE (FEET) =	0.00	"Z" FACTOR =	2.500
MANNING'S FACTOR =	0.045	MAXIMUM DEPTH (FEET) =	3.00
CHANNEL FLOW THRU SUBAREA (CFS) =	106.12		
FLOW VELOCITY (FEET/SEC.) =	10.36	FLOW DEPTH (FEET) =	2.02
TRAVEL TIME (MIN.) =	1.50	Tc (MIN.) =	15.71
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20125.00 =	3064.17	FEET.	

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20125.00 TO NODE 20125.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) =	15.71				
* 100 YEAR RAINFALL INTENSITY (INCH/HR) =	2.793				
SUBAREA LOSS RATE DATA (AMC II):					
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
NATURAL FAIR COVER					
"OPEN BRUSH"	B	36.51	0.61	1.000	66
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) =	0.61				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	1.000				
SUBAREA AREA (ACRES) =	36.51	SUBAREA RUNOFF (CFS) =	71.59		
EFFECTIVE AREA (ACRES) =	86.64	AREA-AVERAGED Fm (INCH/HR) =	0.61		
AREA-AVERAGED Fp (INCH/HR) =	0.61	AREA-AVERAGED Ap =	1.00		
TOTAL AREA (ACRES) =	86.6	PEAK FLOW RATE (CFS) =	169.90		

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20125.00 TO NODE 20126.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 2620.00 DOWNSTREAM(FEET) = 2600.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 1333.93 CHANNEL SLOPE = 0.0150  
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 2.500  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 4.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 169.90  
FLOW VELOCITY(FEET/SEC.) = 5.58 FLOW DEPTH(FEET) = 3.49  
TRAVEL TIME(MIN.) = 3.98 Tc(MIN.) = 19.70  
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20126.00 = 4398.10 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20126.00 TO NODE 20126.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 19.70  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.439  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
NATURAL FAIR COVER  
"OPEN BRUSH" B 60.59 0.61 1.000 66  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.61  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000  
SUBAREA AREA(ACRES) = 60.59 SUBAREA RUNOFF(CFS) = 99.51  
EFFECTIVE AREA(ACRES) = 147.23 AREA-AVERAGED Fm(INCH/HR) = 0.61  
AREA-AVERAGED Fp(INCH/HR) = 0.61 AREA-AVERAGED Ap = 1.00  
TOTAL AREA(ACRES) = 147.2 PEAK FLOW RATE(CFS) = 241.79

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20126.00 TO NODE 20127.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 2600.00 DOWNSTREAM(FEET) = 2420.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 1404.24 CHANNEL SLOPE = 0.1282  
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 2.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 10.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 241.79  
FLOW VELOCITY(FEET/SEC.) = 11.03 FLOW DEPTH(FEET) = 1.00  
TRAVEL TIME(MIN.) = 2.12 Tc(MIN.) = 21.82  
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20127.00 = 5802.34 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20127.00 TO NODE 20127.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 21.82  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.294  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
NATURAL FAIR COVER  
"OPEN BRUSH" B 45.37 0.61 1.000 66  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.61  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000  
SUBAREA AREA(ACRES) = 45.37 SUBAREA RUNOFF(CFS) = 68.58  
EFFECTIVE AREA(ACRES) = 192.60 AREA-AVERAGED Fm(INCH/HR) = 0.61  
AREA-AVERAGED Fp(INCH/HR) = 0.61 AREA-AVERAGED Ap = 1.00  
TOTAL AREA(ACRES) = 192.6 PEAK FLOW RATE(CFS) = 291.14

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.18; 6HR = 3.11; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20127.00 TO NODE 20128.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 2420.00 DOWNSTREAM(FEET) = 2240.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 1246.58 CHANNEL SLOPE = 0.1444  
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 2.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 10.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 291.14  
FLOW VELOCITY(FEET/SEC.) = 10.78 FLOW DEPTH(FEET) = 0.85  
TRAVEL TIME(MIN.) = 1.93 Tc(MIN.) = 23.74  
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20128.00 = 7048.92 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20128.00 TO NODE 20128.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 23.74  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.180  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
NATURAL FAIR COVER  
"OPEN BRUSH" B 27.94 0.61 1.000 66  
RESIDENTIAL  
"2 DWELLINGS/ACRE" B 8.51 0.75 0.700 56  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.64  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.930  
SUBAREA AREA(ACRES) = 36.45 SUBAREA RUNOFF(CFS) = 52.07  
EFFECTIVE AREA(ACRES) = 229.05 AREA-AVERAGED Fm(INCH/HR) = 0.61  
AREA-AVERAGED Fp(INCH/HR) = 0.62 AREA-AVERAGED Ap = 0.99  
TOTAL AREA(ACRES) = 229.0 PEAK FLOW RATE(CFS) = 323.52

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

```

*****
FLOW PROCESS FROM NODE 20128.00 TO NODE 20129.00 IS CODE = 54
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 2240.00 DOWNSTREAM(FEET) = 2120.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1393.78 CHANNEL SLOPE = 0.0861
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 10.00
CHANNEL FLOW THRU SUBAREA(CFS) = 323.52
FLOW VELOCITY(FEET/SEC.) = 9.54 FLOW DEPTH(FEET) = 1.06
TRAVEL TIME(MIN.) = 2.43 Tc(MIN.) = 26.18
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20129.00 = 8442.70 FEET.

```

```

*****
FLOW PROCESS FROM NODE 20129.00 TO NODE 20129.00 IS CODE = 81
-----

```

```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
MAINLINE Tc(MIN.) = 26.18
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.056
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE           GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
NATURAL FAIR COVER
"OPEN BRUSH"       B       18.57   0.61   1.000   66
RESIDENTIAL
"2 DWELLINGS/ACRE" B       10.38   0.75   0.700   56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.65
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.892
SUBAREA AREA(ACRES) = 28.95 SUBAREA RUNOFF(CFS) = 38.42
EFFECTIVE AREA(ACRES) = 258.00 AREA-AVERAGED Fm(INCH/HR) = 0.61
AREA-AVERAGED Fp(INCH/HR) = 0.62 AREA-AVERAGED Ap = 0.98
TOTAL AREA(ACRES) = 258.0 PEAK FLOW RATE(CFS) = 336.38

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

```

```

*****
FLOW PROCESS FROM NODE 10129.00 TO NODE 20130.00 IS CODE = 54
-----

```

```

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 2120.00 DOWNSTREAM(FEET) = 1995.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 2018.40 CHANNEL SLOPE = 0.0619
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 10.00
CHANNEL FLOW THRU SUBAREA(CFS) = 336.38
FLOW VELOCITY(FEET/SEC.) = 8.71 FLOW DEPTH(FEET) = 1.19
TRAVEL TIME(MIN.) = 3.86 Tc(MIN.) = 30.04
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20130.00 = 10461.10 FEET.

```

```

*****
FLOW PROCESS FROM NODE 20130.00 TO NODE 20130.00 IS CODE = 81
-----

```

```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
MAINLINE Tc(MIN.) = 30.04
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.893
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE           GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
".4 DWELLING/ACRE" B       28.04   0.75   0.900   56
NATURAL FAIR COVER
"OPEN BRUSH"       B       51.49   0.61   1.000   66
RESIDENTIAL
"2 DWELLINGS/ACRE" B       30.71   0.75   0.700   56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.68
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.891
SUBAREA AREA(ACRES) = 110.24 SUBAREA RUNOFF(CFS) = 127.90
EFFECTIVE AREA(ACRES) = 368.24 AREA-AVERAGED Fm(INCH/HR) = 0.61
AREA-AVERAGED Fp(INCH/HR) = 0.64 AREA-AVERAGED Ap = 0.95
TOTAL AREA(ACRES) = 368.2 PEAK FLOW RATE(CFS) = 426.43

```

```

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.07; 6HR = 2.84; 24HR = 7.50

```

```

*****
FLOW PROCESS FROM NODE 20130.00 TO NODE 20148.00 IS CODE = 54
-----

```

```

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 1995.00 DOWNSTREAM(FEET) = 1925.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1246.14 CHANNEL SLOPE = 0.0562
CHANNEL BASE(FEET) = 30.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 10.00
CHANNEL FLOW THRU SUBAREA(CFS) = 426.43
FLOW VELOCITY(FEET/SEC.) = 9.23 FLOW DEPTH(FEET) = 1.41
TRAVEL TIME(MIN.) = 2.25 Tc(MIN.) = 32.29
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20148.00 = 11707.24 FEET.

```

```

*****
FLOW PROCESS FROM NODE 20148.00 TO NODE 20148.00 IS CODE = 81
-----

```

```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
MAINLINE Tc(MIN.) = 32.29
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.813
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE           GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
".4 DWELLING/ACRE" B       19.93   0.75   0.900   56
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B       0.65   0.75   0.600   56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.891
SUBAREA AREA(ACRES) = 20.58 SUBAREA RUNOFF(CFS) = 21.24
EFFECTIVE AREA(ACRES) = 388.82 AREA-AVERAGED Fm(INCH/HR) = 0.61
AREA-AVERAGED Fp(INCH/HR) = 0.64 AREA-AVERAGED Ap = 0.95
TOTAL AREA(ACRES) = 388.8 PEAK FLOW RATE(CFS) = 426.43

```

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20148.00 TO NODE 20148.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS =	2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:	
TIME OF CONCENTRATION(MIN.) =	32.29
RAINFALL INTENSITY(INCH/HR) =	1.81
AREA-AVERAGED Fm(INCH/HR) =	0.61
AREA-AVERAGED Fp(INCH/HR) =	0.64
AREA-AVERAGED Ap =	0.95
EFFECTIVE STREAM AREA(ACRES) =	388.82
TOTAL STREAM AREA(ACRES) =	388.82
PEAK FLOW RATE(CFS) AT CONFLUENCE =	426.43

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20140.00 TO NODE 20141.00 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) =	823.61
ELEVATION DATA: UPSTREAM(FEET) =	3000.00
DOWNSTREAM(FEET) =	2690.00

Tc = K*[ (LENGTH** 3.00) / (ELEVATION CHANGE) ]**0.20						
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =	12.588					
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	3.190					
SUBAREA Tc AND LOSS RATE DATA(AMC II):						
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS	Tc
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN	(MIN.)
NATURAL FAIR COVER						
"OPEN BRUSH"	B	8.14	0.61	1.000	66	12.59
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =	0.61					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	1.000					
SUBAREA RUNOFF(CFS) =	18.87					
TOTAL AREA(ACRES) =	8.14					
PEAK FLOW RATE(CFS) =	18.87					

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.31; 30M = 0.64; 1HR = 0.85; 3HR = 1.53; 6HR = 2.22; 24HR = 4.71

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20141.00 TO NODE 20142.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	2690.00
DOWNSTREAM(FEET) =	2560.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	610.78
CHANNEL SLOPE =	0.2128
CHANNEL BASE(FEET) =	0.00
"Z" FACTOR =	2.500
MANNING'S FACTOR =	0.045
MAXIMUM DEPTH(FEET) =	2.00
CHANNEL FLOW THRU SUBAREA(CFS) =	18.87
FLOW VELOCITY(FEET/SEC.) =	8.66
FLOW DEPTH(FEET) =	0.93

TRAVEL TIME(MIN.) = 1.18 Tc(MIN.) = 13.76  
LONGEST FLOWPATH FROM NODE 20140.00 TO NODE 20142.00 = 1434.39 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20142.00 TO NODE 20142.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) =	13.76				
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	3.024				
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
NATURAL FAIR COVER					
"OPEN BRUSH"	B	15.44	0.61	1.000	66
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =	0.61				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	1.000				
SUBAREA AREA(ACRES) =	15.44				
SUBAREA RUNOFF(CFS) =	33.49				
EFFECTIVE AREA(ACRES) =	23.58				
AREA-AVERAGED Fm(INCH/HR) =	0.61				
AREA-AVERAGED Fp(INCH/HR) =	0.61				
AREA-AVERAGED Ap =	1.00				
TOTAL AREA(ACRES) =	23.6				
PEAK FLOW RATE(CFS) =	51.14				

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20142.00 TO NODE 20143.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	2560.00
DOWNSTREAM(FEET) =	2420.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	771.13
CHANNEL SLOPE =	0.1816
CHANNEL BASE(FEET) =	0.00
"Z" FACTOR =	2.500
MANNING'S FACTOR =	0.045
MAXIMUM DEPTH(FEET) =	2.00
CHANNEL FLOW THRU SUBAREA(CFS) =	51.14
FLOW VELOCITY(FEET/SEC.) =	10.54
FLOW DEPTH(FEET) =	1.39
TRAVEL TIME(MIN.) =	1.22
Tc(MIN.) =	14.98
LONGEST FLOWPATH FROM NODE 20140.00 TO NODE 20143.00 =	2205.52 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20143.00 TO NODE 20143.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) =	14.98				
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	2.874				
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
NATURAL FAIR COVER					
"OPEN BRUSH"	B	22.70	0.61	1.000	66
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =	0.61				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	1.000				
SUBAREA AREA(ACRES) =	22.70				
SUBAREA RUNOFF(CFS) =	46.16				
EFFECTIVE AREA(ACRES) =	46.28				
AREA-AVERAGED Fm(INCH/HR) =	0.61				
AREA-AVERAGED Fp(INCH/HR) =	0.61				
AREA-AVERAGED Ap =	1.00				
TOTAL AREA(ACRES) =	46.3				
PEAK FLOW RATE(CFS) =	94.12				

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20143.00 TO NODE 20144.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	2420.00	DOWNSTREAM(FEET) =	2240.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	1310.58	CHANNEL SLOPE =	0.1373
CHANNEL BASE(FEET) =	0.00	"Z" FACTOR =	2.500
MANNING'S FACTOR =	0.045	MAXIMUM DEPTH(FEET) =	2.00
CHANNEL FLOW THRU SUBAREA(CFS) =	94.12		
FLOW VELOCITY(FEET/SEC.) =	11.01	FLOW DEPTH(FEET) =	1.85
TRAVEL TIME(MIN.) =	1.98	Tc(MIN.) =	16.97
LONGEST FLOWPATH FROM NODE 20140.00 TO NODE 20144.00 =	3516.10	FEET.	

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20144.00 TO NODE 20144.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) =	16.97				
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	2.667				
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER					
"OPEN BRUSH"	B	61.27	0.61	1.000	66
RESIDENTIAL					
".4 DWELLING/ACRE"	B	11.25	0.75	0.900	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =	0.63				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	0.984				
SUBAREA AREA(ACRES) =	72.52	SUBAREA RUNOFF(CFS) =	133.40		
EFFECTIVE AREA(ACRES) =	118.80	AREA-AVERAGED Fm(INCH/HR) =	0.62		
AREA-AVERAGED Fp(INCH/HR) =	0.63	AREA-AVERAGED Ap =	0.99		
TOTAL AREA(ACRES) =	118.8	PEAK FLOW RATE(CFS) =	218.91		

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.25; 6HR = 3.25; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20144.00 TO NODE 20145.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	2240.00	DOWNSTREAM(FEET) =	2150.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	1185.29	CHANNEL SLOPE =	0.0759
CHANNEL BASE(FEET) =	5.00	"Z" FACTOR =	2.000
MANNING'S FACTOR =	0.045	MAXIMUM DEPTH(FEET) =	2.50
CHANNEL FLOW THRU SUBAREA(CFS) =	218.91		
FLOW VELOCITY(FEET/SEC.) =	11.15	FLOW DEPTH(FEET) =	2.12
TRAVEL TIME(MIN.) =	1.77	Tc(MIN.) =	18.74
LONGEST FLOWPATH FROM NODE 20140.00 TO NODE 20145.00 =	4701.39	FEET.	

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20145.00 TO NODE 20145.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) =	18.74				
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	2.513				
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER					
"OPEN BRUSH"	B	27.90	0.61	1.000	66
RESIDENTIAL					
".4 DWELLING/ACRE"	B	18.45	0.75	0.900	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =	0.66				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	0.960				
SUBAREA AREA(ACRES) =	46.35	SUBAREA RUNOFF(CFS) =	78.22		
EFFECTIVE AREA(ACRES) =	165.15	AREA-AVERAGED Fm(INCH/HR) =	0.62		
AREA-AVERAGED Fp(INCH/HR) =	0.64	AREA-AVERAGED Ap =	0.98		
TOTAL AREA(ACRES) =	165.1	PEAK FLOW RATE(CFS) =	280.63		

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.15; 6HR = 3.04; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20145.00 TO NODE 20146.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	2150.00	DOWNSTREAM(FEET) =	2065.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	1106.66	CHANNEL SLOPE =	0.0768
CHANNEL BASE(FEET) =	5.00	"Z" FACTOR =	2.000
MANNING'S FACTOR =	0.045	MAXIMUM DEPTH(FEET) =	3.00
CHANNEL FLOW THRU SUBAREA(CFS) =	280.63		
FLOW VELOCITY(FEET/SEC.) =	11.94	FLOW DEPTH(FEET) =	2.40
TRAVEL TIME(MIN.) =	1.54	Tc(MIN.) =	20.28
LONGEST FLOWPATH FROM NODE 20140.00 TO NODE 20146.00 =	5808.05	FEET.	

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20146.00 TO NODE 20146.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) =	20.28				
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	2.396				
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER					
"OPEN BRUSH"	B	5.66	0.61	1.000	66
RESIDENTIAL					
".4 DWELLING/ACRE"	B	28.22	0.75	0.900	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =	0.72				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	0.917				
SUBAREA AREA(ACRES) =	33.88	SUBAREA RUNOFF(CFS) =	52.84		
EFFECTIVE AREA(ACRES) =	199.03	AREA-AVERAGED Fm(INCH/HR) =	0.63		
AREA-AVERAGED Fp(INCH/HR) =	0.65	AREA-AVERAGED Ap =	0.97		

TOTAL AREA (ACRES) = 199.0 PEAK FLOW RATE (CFS) = 316.14

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*

FLOW PROCESS FROM NODE 20146.00 TO NODE 20147.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 2065.00 DOWNSTREAM (FEET) = 1980.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 1084.55 CHANNEL SLOPE = 0.0784  
CHANNEL BASE (FEET) = 5.00 "Z" FACTOR = 2.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH (FEET) = 3.00  
CHANNEL FLOW THRU SUBAREA (CFS) = 316.14  
FLOW VELOCITY (FEET/SEC.) = 12.43 FLOW DEPTH (FEET) = 2.53  
TRAVEL TIME (MIN.) = 1.45 Tc (MIN.) = 21.74  
LONGEST FLOWPATH FROM NODE 20140.00 TO NODE 20147.00 = 6892.60 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20147.00 TO NODE 20147.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 21.74  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.299  
SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
".4 DWELLING/ACRE" B 15.70 0.75 0.900 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.75  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.900  
SUBAREA AREA (ACRES) = 15.70 SUBAREA RUNOFF (CFS) = 22.97  
EFFECTIVE AREA (ACRES) = 214.73 AREA-AVERAGED Fm (INCH/HR) = 0.63  
AREA-AVERAGED Fp (INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.97  
TOTAL AREA (ACRES) = 214.7 PEAK FLOW RATE (CFS) = 321.65

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*

FLOW PROCESS FROM NODE 20147.00 TO NODE 20148.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 1980.00 DOWNSTREAM (FEET) = 1925.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 934.91 CHANNEL SLOPE = 0.0588  
CHANNEL BASE (FEET) = 5.00 "Z" FACTOR = 2.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH (FEET) = 3.00  
CHANNEL FLOW THRU SUBAREA (CFS) = 321.65  
FLOW VELOCITY (FEET/SEC.) = 11.24 FLOW DEPTH (FEET) = 2.73  
TRAVEL TIME (MIN.) = 1.39 Tc (MIN.) = 23.12  
LONGEST FLOWPATH FROM NODE 20140.00 TO NODE 20148.00 = 7827.51 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20148.00 TO NODE 20148.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 23.12  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.215  
SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
".4 DWELLING/ACRE" B 14.97 0.75 0.900 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.75  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.900  
SUBAREA AREA (ACRES) = 14.97 SUBAREA RUNOFF (CFS) = 20.77  
EFFECTIVE AREA (ACRES) = 229.70 AREA-AVERAGED Fm (INCH/HR) = 0.64  
AREA-AVERAGED Fp (INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.96  
TOTAL AREA (ACRES) = 229.7 PEAK FLOW RATE (CFS) = 326.24

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*

FLOW PROCESS FROM NODE 20148.00 TO NODE 20148.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION (MIN.) = 23.12  
RAINFALL INTENSITY (INCH/HR) = 2.21  
AREA-AVERAGED Fm (INCH/HR) = 0.64  
AREA-AVERAGED Fp (INCH/HR) = 0.66  
AREA-AVERAGED Ap = 0.96  
EFFECTIVE STREAM AREA (ACRES) = 229.70  
TOTAL STREAM AREA (ACRES) = 229.70  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 326.24

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	426.43	32.29	1.813	0.64 (0.61)	0.95	388.8	20120.00
2	326.24	23.12	2.215	0.66 (0.64)	0.96	229.7	20140.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	733.66	23.12	2.215	0.65 (0.62)	0.95	508.1	20140.00
2	669.52	32.29	1.813	0.65 (0.62)	0.95	618.5	20120.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
PEAK FLOW RATE (CFS) = 733.66 Tc (MIN.) = 23.12  
EFFECTIVE AREA (ACRES) = 508.11 AREA-AVERAGED Fm (INCH/HR) = 0.62  
AREA-AVERAGED Fp (INCH/HR) = 0.65 AREA-AVERAGED Ap = 0.95  
TOTAL AREA (ACRES) = 618.5

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20148.00 = 11707.24 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20148.00 TO NODE 20149.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1925.00 DOWNSTREAM(FEET) = 1900.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 764.60 CHANNEL SLOPE = 0.0327
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 5.00
CHANNEL FLOW THRU SUBAREA(CFS) = 733.66
FLOW VELOCITY(FEET/SEC.) = 10.95 FLOW DEPTH(FEET) = 3.80
TRAVEL TIME(MIN.) = 1.16 Tc(MIN.) = 24.29
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20149.00 = 12471.84 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20149.00 TO NODE 20149.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 24.29
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.151
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
".4 DWELLING/ACRE" B 20.34 0.75 0.900 56
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B 0.62 0.75 0.600 56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.891
SUBAREA AREA(ACRES) = 20.96 SUBAREA RUNOFF(CFS) = 28.00
EFFECTIVE AREA(ACRES) = 529.07 AREA-AVERAGED Fm(INCH/HR) = 0.62
AREA-AVERAGED Fp(INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.95
TOTAL AREA(ACRES) = 639.5 PEAK FLOW RATE(CFS) = 733.66
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*

FLOW PROCESS FROM NODE 20149.00 TO NODE 20150.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1900.00 DOWNSTREAM(FEET) = 1850.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1212.57 CHANNEL SLOPE = 0.0412
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 5.00
CHANNEL FLOW THRU SUBAREA(CFS) = 733.66
FLOW VELOCITY(FEET/SEC.) = 11.90 FLOW DEPTH(FEET) = 3.59
TRAVEL TIME(MIN.) = 1.70 Tc(MIN.) = 25.99
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20150.00 = 13684.41 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20150.00 TO NODE 20150.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 25.99
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.065
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
".4 DWELLING/ACRE" B 8.58 0.75 0.900 56
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B 0.10 0.75 0.600 56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.897
SUBAREA AREA(ACRES) = 8.68 SUBAREA RUNOFF(CFS) = 10.89
EFFECTIVE AREA(ACRES) = 537.75 AREA-AVERAGED Fm(INCH/HR) = 0.62
AREA-AVERAGED Fp(INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.95
TOTAL AREA(ACRES) = 648.2 PEAK FLOW RATE(CFS) = 733.66
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*

FLOW PROCESS FROM NODE 20150.00 TO NODE 20150.00 IS CODE = 71

>>>>PEAK FLOW RATE ESTIMATOR CHANGED TO UNIT-HYDROGRAPH METHOD<<<<<
>>>>USING TIME-OF-CONCENTRATION OF LONGEST FLOWPATH<<<<<

UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.46;30M= 0.95;1H= 1.24;3H= 2.14;6H= 3.00;24H= 7.46
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.59; LAG(HR) = 0.47; Fm(INCH/HR) = 0.62; Ybar = 0.54
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;
3HR = 1.00; 6HR = 1.00; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 648.2
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20150.00 = 13684.41 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0489; Lca/L=0.4,n=.0438; Lca/L=0.5,n=.0402;Lca/L=0.6,n=.0375
TIME OF PEAK FLOW(HR) = 16.50 RUNOFF VOLUME(AF) = 197.89
UNIT-HYDROGRAPH METHOD PEAK FLOW RATE(CFS) = 747.48
TOTAL PEAK FLOW RATE(CFS) = 747.48 (SOURCE FLOW INCLUDED)
RATIONAL METHOD PEAK FLOW RATE(CFS) = 733.66
(UPSTREAM NODE PEAK FLOW RATE(CFS) = 733.66)
PEAK FLOW RATE(CFS) USED = 747.48

\*\*\*\*\*

FLOW PROCESS FROM NODE 20150.00 TO NODE 20150.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

\*\* MAIN STREAM CONFLUENCE DATA \*\*
PEAK FLOW RATE(CFS) = 747.48 Tc(MIN.) = 35.23
AREA-AVERAGED Fm(INCH/HR) = 0.62 Ybar = 0.54

TOTAL AREA (ACRES) = 648.2  
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20150.00 = 13684.41 FEET.

\*\* MEMORY BANK # 1 CONFLUENCE DATA \*\*

PEAK FLOW RATE (CFS) = 1525.78 Tc (MIN.) = 24.67  
AREA-AVERAGED Fm (INCH/HR) = 0.60 Ybar = 0.53  
TOTAL AREA (ACRES) = 1052.3  
LONGEST FLOWPATH FROM NODE 20100.00 TO NODE 20150.00 = 13659.16 FEET.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M= 0.46; 30M= 0.95; 1H= 1.25; 3H= 2.19; 6H= 3.11; 24H= 7.51

S-GRAPH: VALLEY (DEV.)=100.0%; VALLEY (UNDEV.)/DESERT= 0.0%

MOUNTAIN= 0.0%; FOOTHILL= 0.0%; DESERT (UNDEV.)= 0.0%

Tc (HR) = 0.59; LAG (HR) = 0.47; Fm (INCH/HR) = 0.61; Ybar = 0.53

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.92; 30M = 0.92; 1HR = 0.92;

3HR = 0.99; 6HR = 0.99; 24HR = 1.00

UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 1700.5

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20150.00 = 13684.41 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3, n=.0489; Lca/L=0.4, n=.0438; Lca/L=0.5, n=.0402; Lca/L=0.6, n=.0375

TIME OF PEAK FLOW (HR) = 16.50 RUNOFF VOLUME (AF) = 524.96

PEAK FLOW RATE (CFS) = 1868.80

\*\*\*\*\*

FLOW PROCESS FROM NODE 20150.00 TO NODE 20150.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<

\*\*\*\*\*

FLOW PROCESS FROM NODE 20150.00 TO NODE 20151.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM (FEET) = 1850.00 DOWNSTREAM (FEET) = 1785.00

CHANNEL LENGTH THRU SUBAREA (FEET) = 1753.77 CHANNEL SLOPE = 0.0371

CHANNEL BASE (FEET) = 10.00 "Z" FACTOR = 2.000

MANNING'S FACTOR = 0.035 MAXIMUM DEPTH (FEET) = 10.00

CHANNEL FLOW THRU SUBAREA (CFS) = 1868.80

FLOW VELOCITY (FEET/SEC.) = 17.69 FLOW DEPTH (FEET) = 5.19

TRAVEL TIME (MIN.) = 1.65 Tc (MIN.) = 36.88

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20151.00 = 15438.18 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20151.00 TO NODE 20151.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

MAINLINE Tc (MIN.) = 36.88

\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.674

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL ".4 DWELLING/ACRE"	B	24.58	0.75	0.900	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.75

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900

SUBAREA AREA (ACRES) = 24.58

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M= 0.46; 30M= 0.95; 1H= 1.25; 3H= 2.18; 6H= 3.11; 24H= 7.51

S-GRAPH: VALLEY (DEV.)=100.0%; VALLEY (UNDEV.)/DESERT= 0.0%

MOUNTAIN= 0.0%; FOOTHILL= 0.0%; DESERT (UNDEV.)= 0.0%

Tc (HR) = 0.61; LAG (HR) = 0.49; Fm (INCH/HR) = 0.61; Ybar = 0.53

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.92; 30M = 0.92; 1HR = 0.92;

3HR = 0.99; 6HR = 0.99; 24HR = 1.00

UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 1725.0

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20151.00 = 15438.18 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3, n=.0460; Lca/L=0.4, n=.0413; Lca/L=0.5, n=.0379; Lca/L=0.6, n=.0354

TIME OF PEAK FLOW (HR) = 16.58 RUNOFF VOLUME (AF) = 531.22

UNIT-HYDROGRAPH PEAK FLOW RATE (CFS) = 1822.13

TOTAL AREA (ACRES) = 1725.0 PEAK FLOW RATE (CFS) = 1868.80

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.33

\*\*\*\*\*

FLOW PROCESS FROM NODE 20151.00 TO NODE 20151.00 IS CODE = 152

>>>>STORE PEAK FLOWRATE TABLE TO A FILE<<<<

PEAK FLOWRATE TABLE FILE NAME: 20151.DNA

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 1725.0 TC (MIN.) = 36.88

AREA-AVERAGED Fm (INCH/HR) = 0.61 Ybar = 0.53

PEAK FLOW RATE (CFS) = 1868.80

END OF INTEGRATED RATIONAL/UNIT-HYDROGRAPH METHOD ANALYSIS





\*\*\*\*\*  
RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
(Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)  
(c) Copyright 1983-2013 Advanced Engineering Software (aes)  
Ver. 20.0 Release Date: 06/01/2013 License ID 1264

Analysis prepared by:

\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* REDLANDS MPD - UPDATE \*  
\* REVISED RATIONAL METHOD HYDROLOGY - TO NODE 20274 \*  
\* BY TMULI SEPT 2013 \*  
\*\*\*\*\*

FILE NAME: LR0202.DAT  
TIME/DATE OF STUDY: 15:31 09/24/2013

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:  
-----  
--\*TIME-OF-CONCENTRATION MODEL\*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00  
SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00  
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85  
\*USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL\*

SLOPE OF INTENSITY DURATION CURVE(LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000  
USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.2500

\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD\*

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	18.0	12.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
2	20.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
3	22.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
4	15.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
5	18.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
6	15.0	10.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
7	16.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
8	16.0	10.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
9	17.0	10.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
10	30.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
11	24.0	15.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
12	24.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
13	32.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
14	39.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
15	36.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
16	12.5	5.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180

17	20.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
18	26.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
19	52.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:  
1. Relative Flow-Depth = 0.20 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)  
2. (Depth)\*(Velocity)\*Constraint = 6.0 (FT\*FT/S)  
\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*  
\*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

UNIT-HYDROGRAPH MODEL SELECTIONS/PARAMETERS:  
WATERSHED LAG = 0.80 \* Tc  
USED "VALLEY UNDEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 1 UNITS/ACRE AND LESS; AND "VALLEY DEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 2 UNITS/ACRE AND MORE.  
PRECIPITATION DATA ENTERED ON SUBAREA BASIS.  
SIERRA MADRE DEPTH-AREA FACTORS USED.  
\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR UNIT HYDROGRAPH METHOD\*

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20200.00 TO NODE 20201.00 IS CODE = 21

-----  
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<  
-----  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 508.83  
ELEVATION DATA: UPSTREAM(FEET) = 1945.00 DOWNSTREAM(FEET) = 1935.00

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20  
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.936  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.471  
SUBAREA Tc AND LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 4.64 0.98 0.600 32 10.94  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.600  
SUBAREA RUNOFF(CFS) = 12.05  
TOTAL AREA(ACRES) = 4.64 PEAK FLOW RATE(CFS) = 12.05

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.12; 6HR = 2.96; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20201.00 TO NODE 20202.00 IS CODE = 92

-----  
>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<<<  
-----  
UPSTREAM NODE ELEVATION(FEET) = 1935.00  
DOWNSTREAM NODE ELEVATION(FEET) = 1930.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 620.72  
"V" GUTTER WIDTH(FEET) = 3.00 GUTTER HIKE(FEET) = 0.250  
PAVEMENT LIP(FEET) = 0.100 MANNING'S N = .0150  
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.01700

MAXIMUM DEPTH (FEET) = 1.00  
 \* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.961  
 SUBAREA LOSS RATE DATA (AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	6.32	0.98	0.600	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.98  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.600  
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 18.81  
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 3.12  
 AVERAGE FLOW DEPTH (FEET) = 0.63 FLOOD WIDTH (FEET) = 35.63  
 "V" GUTTER FLOW TRAVEL TIME (MIN.) = 3.32 Tc (MIN.) = 14.25  
 SUBAREA AREA (ACRES) = 6.32 SUBAREA RUNOFF (CFS) = 13.51  
 EFFECTIVE AREA (ACRES) = 10.96 AREA-AVERAGED Fm (INCH/HR) = 0.59  
 AREA-AVERAGED Fp (INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.60  
 TOTAL AREA (ACRES) = 11.0 PEAK FLOW RATE (CFS) = 23.44

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.76; 24HR = 7.50  
  
 END OF SUBAREA "V" GUTTER HYDRAULICS:  
 DEPTH (FEET) = 0.66 FLOOD WIDTH (FEET) = 39.52  
 FLOW VELOCITY (FEET/SEC.) = 3.22 DEPTH\*VELOCITY (FT\*FT/SEC) = 2.13  
 LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20202.00 = 1129.55 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20202.00 TO NODE 20203.00 IS CODE = 62

>>>> COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA <<<<<<  
 >>>> (STREET TABLE SECTION # 13 USED) <<<<<<

=====  
 UPSTREAM ELEVATION (FEET) = 1930.00 DOWNSTREAM ELEVATION (FEET) = 1910.00  
 STREET LENGTH (FEET) = 369.50 CURB HEIGHT (INCHES) = 8.0  
 STREET HALFWIDTH (FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 20.00  
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 34.88  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH (FEET) = 0.49  
 HALFSTREET FLOOD WIDTH (FEET) = 16.57  
 AVERAGE FLOW VELOCITY (FEET/SEC.) = 5.94  
 PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 2.91  
 STREET FLOW TRAVEL TIME (MIN.) = 1.04 Tc (MIN.) = 15.29  
 \* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.839

SUBAREA LOSS RATE DATA (AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	11.02	0.98	0.600	32

MOBILE HOME PARK A 0.23 0.98 0.250 32  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.98  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.593  
 SUBAREA AREA (ACRES) = 11.25 SUBAREA RUNOFF (CFS) = 22.89  
 EFFECTIVE AREA (ACRES) = 22.21 AREA-AVERAGED Fm (INCH/HR) = 0.58  
 AREA-AVERAGED Fp (INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.60  
 TOTAL AREA (ACRES) = 22.2 PEAK FLOW RATE (CFS) = 45.12

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH (FEET) = 0.53 HALFSTREET FLOOD WIDTH (FEET) = 18.37  
 FLOW VELOCITY (FEET/SEC.) = 6.33 DEPTH\*VELOCITY (FT\*FT/SEC.) = 3.33  
 LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20203.00 = 1499.05 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20203.00 TO NODE 20204.00 IS CODE = 62

>>>> COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA <<<<<<  
 >>>> (STREET TABLE SECTION # 13 USED) <<<<<<

=====  
 UPSTREAM ELEVATION (FEET) = 1910.00 DOWNSTREAM ELEVATION (FEET) = 1895.00  
 STREET LENGTH (FEET) = 418.06 CURB HEIGHT (INCHES) = 8.0  
 STREET HALFWIDTH (FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 20.00  
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 58.61  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH (FEET) = 0.60  
 HALFSTREET FLOOD WIDTH (FEET) = 22.12  
 AVERAGE FLOW VELOCITY (FEET/SEC.) = 5.77  
 PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 3.46  
 STREET FLOW TRAVEL TIME (MIN.) = 1.21 Tc (MIN.) = 16.50

\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.712  
 SUBAREA LOSS RATE DATA (AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	6.00	0.98	0.600	32
MOBILE HOME PARK	A	6.97	0.98	0.250	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.97  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.412  
 SUBAREA AREA (ACRES) = 12.97 SUBAREA RUNOFF (CFS) = 26.97  
 EFFECTIVE AREA (ACRES) = 35.18 AREA-AVERAGED Fm (INCH/HR) = 0.52  
 AREA-AVERAGED Fp (INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.53  
 TOTAL AREA (ACRES) = 35.2 PEAK FLOW RATE (CFS) = 69.56

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 0.63 HALFSTREET FLOOD WIDTH(FEET) = 23.68  
FLOW VELOCITY(FEET/SEC.) = 6.00 DEPTH\*VELOCITY(FT\*FT/SEC.) = 3.79  
LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20204.00 = 1917.11 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20204.00 TO NODE 20205.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 13 USED)<<<<<  
-----

UPSTREAM ELEVATION(FEET) = 1895.00 DOWNSTREAM ELEVATION(FEET) = 1875.00  
STREET LENGTH(FEET) = 555.00 CURB HEIGHT(INCHES) = 8.0  
STREET HALFWIDTH(FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 86.00  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 0.67  
HALFSTREET FLOOD WIDTH(FEET) = 26.00  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.32  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 4.25

STREET FLOW TRAVEL TIME(MIN.) = 1.46 Tc(MIN.) = 17.96  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.577

SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 8.31 0.98 0.600 32  
MOBILE HOME PARK A 8.55 0.98 0.250 32  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.423  
SUBAREA AREA(ACRES) = 16.86 SUBAREA RUNOFF(CFS) = 32.86  
EFFECTIVE AREA(ACRES) = 52.04 AREA-AVERAGED Fm(INCH/HR) = 0.48  
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.49  
TOTAL AREA(ACRES) = 52.0 PEAK FLOW RATE(CFS) = 98.16

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 0.70 HALFSTREET FLOOD WIDTH(FEET) = 28.66  
FLOW VELOCITY(FEET/SEC.) = 6.52 DEPTH\*VELOCITY(FT\*FT/SEC.) = 4.56  
LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20205.00 = 2472.11 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20205.00 TO NODE 20206.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>(STREET TABLE SECTION # 13 USED)<<<<<  
-----

UPSTREAM ELEVATION(FEET) = 1875.00 DOWNSTREAM ELEVATION(FEET) = 1855.00  
STREET LENGTH(FEET) = 568.00 CURB HEIGHT(INCHES) = 8.0  
STREET HALFWIDTH(FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 104.12  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 0.71  
HALFSTREET FLOOD WIDTH(FEET) = 30.06  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.56  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 4.68  
STREET FLOW TRAVEL TIME(MIN.) = 1.44 Tc(MIN.) = 19.41  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.461

SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
MOBILE HOME PARK A 4.58 0.98 0.250 32  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 1.65 0.98 0.600 32  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.343  
SUBAREA AREA(ACRES) = 6.23 SUBAREA RUNOFF(CFS) = 11.92  
EFFECTIVE AREA(ACRES) = 58.27 AREA-AVERAGED Fm(INCH/HR) = 0.47  
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.48  
TOTAL AREA(ACRES) = 58.3 PEAK FLOW RATE(CFS) = 104.61

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 0.71 HALFSTREET FLOOD WIDTH(FEET) = 30.22  
FLOW VELOCITY(FEET/SEC.) = 6.55 DEPTH\*VELOCITY(FT\*FT/SEC.) = 4.68  
LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20206.00 = 3040.11 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20206.00 TO NODE 20214.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 13 USED)<<<<<  
-----

UPSTREAM ELEVATION(FEET) = 1855.00 DOWNSTREAM ELEVATION(FEET) = 1840.00  
STREET LENGTH(FEET) = 411.00 CURB HEIGHT(INCHES) = 8.0  
STREET HALFWIDTH(FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 106.73  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 0.71  
HALFSTREET FLOOD WIDTH(FEET) = 30.22  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.68  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 4.77  
STREET FLOW TRAVEL TIME(MIN.) = 1.03 Tc(MIN.) = 20.43  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.386  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
MOBILE HOME PARK A 1.68 0.98 0.250 32  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 0.62 0.98 0.600 32  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.344  
SUBAREA AREA(ACRES) = 2.30 SUBAREA RUNOFF(CFS) = 4.24  
EFFECTIVE AREA(ACRES) = 60.57 AREA-AVERAGED Fm(INCH/HR) = 0.46  
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.47  
TOTAL AREA(ACRES) = 60.6 PEAK FLOW RATE(CFS) = 104.93

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 0.71 HALFSTREET FLOOD WIDTH(FEET) = 29.91  
FLOW VELOCITY(FEET/SEC.) = 6.65 DEPTH\*VELOCITY(FT\*FT/SEC.) = 4.73  
LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20214.00 = 3451.11 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20214.00 TO NODE 20214.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 20.43  
RAINFALL INTENSITY(INCH/HR) = 2.39  
AREA-AVERAGED Fm(INCH/HR) = 0.46  
AREA-AVERAGED Fp(INCH/HR) = 0.97  
AREA-AVERAGED Ap = 0.47  
EFFECTIVE STREAM AREA(ACRES) = 60.57  
TOTAL STREAM AREA(ACRES) = 60.57  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 104.93

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20210.00 TO NODE 20211.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<  
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 940.61  
ELEVATION DATA: UPSTREAM(FEET) = 1875.00 DOWNSTREAM(FEET) = 1850.00

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20  
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.163  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.106  
SUBAREA Tc AND LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 7.95 0.98 0.600 32 13.16  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.600  
SUBAREA RUNOFF(CFS) = 18.04  
TOTAL AREA(ACRES) = 7.95 PEAK FLOW RATE(CFS) = 18.04

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.29

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20211.00 TO NODE 20212.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<  
=====

UPSTREAM ELEVATION(FEET) = 1850.00 DOWNSTREAM ELEVATION(FEET) = 1846.00  
STREET LENGTH(FEET) = 247.17 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 23.83  
\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 0.49  
HALFSTREET FLOOD WIDTH(FEET) = 18.00  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.43  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 1.69  
STREET FLOW TRAVEL TIME(MIN.) = 1.20 Tc(MIN.) = 14.37  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.947

SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 4.82 0.98 0.600 32  
MOBILE HOME PARK A 0.55 0.98 0.250 32  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.564  
SUBAREA AREA(ACRES) = 5.37 SUBAREA RUNOFF(CFS) = 11.59  
EFFECTIVE AREA(ACRES) = 13.32 AREA-AVERAGED Fm(INCH/HR) = 0.57  
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.59  
TOTAL AREA(ACRES) = 13.3 PEAK FLOW RATE(CFS) = 28.49

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.30

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.51 HALFSTREET FLOOD WIDTH(FEET) = 18.68  
FLOW VELOCITY(FEET/SEC.) = 3.69 DEPTH\*VELOCITY(FT\*FT/SEC.) = 1.90  
LONGEST FLOWPATH FROM NODE 20210.00 TO NODE 20212.00 = 1187.78 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20212.00 TO NODE 20213.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<

-----  
UPSTREAM ELEVATION(FEET) = 1846.00 DOWNSTREAM ELEVATION(FEET) = 1843.00  
STREET LENGTH(FEET) = 253.21 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 34.57  
\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.56  
HALFSTREET FLOOD WIDTH(FEET) = 21.19  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.56  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 2.01  
STREET FLOW TRAVEL TIME(MIN.) = 1.19 Tc(MIN.) = 15.55

\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.810

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	2.35	0.98	0.600	32
MOBILE HOME PARK	A	3.23	0.98	0.250	32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.397					
SUBAREA AREA(ACRES) = 5.58 SUBAREA RUNOFF(CFS) = 12.17					
EFFECTIVE AREA(ACRES) = 18.90 AREA-AVERAGED Fm(INCH/HR) = 0.52					
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.53					
TOTAL AREA(ACRES) = 18.9 PEAK FLOW RATE(CFS) = 39.01					

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.33

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.58 HALFSTREET FLOOD WIDTH(FEET) = 22.16  
FLOW VELOCITY(FEET/SEC.) = 3.70 DEPTH\*VELOCITY(FT\*FT/SEC.) = 2.16  
LONGEST FLOWPATH FROM NODE 20210.00 TO NODE 20213.00 = 1440.99 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20213.00 TO NODE 20214.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<

-----  
UPSTREAM ELEVATION(FEET) = 1843.00 DOWNSTREAM ELEVATION(FEET) = 1840.00  
STREET LENGTH(FEET) = 294.25 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 41.41  
\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.61  
HALFSTREET FLOOD WIDTH(FEET) = 23.26  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.58  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 2.17  
STREET FLOW TRAVEL TIME(MIN.) = 1.37 Tc(MIN.) = 16.92  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.672

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	0.63	0.98	0.600	32
MOBILE HOME PARK	A	1.65	0.98	0.250	32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.347					
SUBAREA AREA(ACRES) = 2.28 SUBAREA RUNOFF(CFS) = 4.79					
EFFECTIVE AREA(ACRES) = 21.18 AREA-AVERAGED Fm(INCH/HR) = 0.50					
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.51					
TOTAL AREA(ACRES) = 21.2 PEAK FLOW RATE(CFS) = 41.44					

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.99

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.61 HALFSTREET FLOOD WIDTH(FEET) = 23.26  
FLOW VELOCITY(FEET/SEC.) = 3.59 DEPTH\*VELOCITY(FT\*FT/SEC.) = 2.17  
LONGEST FLOWPATH FROM NODE 20210.00 TO NODE 20214.00 = 1735.24 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20214.00 TO NODE 20214.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

-----  
TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 16.92  
RAINFALL INTENSITY(INCH/HR) = 2.67

AREA-AVERAGED Fm(INCH/HR) = 0.50  
 AREA-AVERAGED Fp(INCH/HR) = 0.98  
 AREA-AVERAGED Ap = 0.51  
 EFFECTIVE STREAM AREA(ACRES) = 21.18  
 TOTAL STREAM AREA(ACRES) = 21.18  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 41.44

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	104.93	20.43	2.386	0.97( 0.46)	0.47	60.6	20200.00
2	41.44	16.92	2.672	0.98( 0.50)	0.51	21.2	20210.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	141.24	16.92	2.672	0.97( 0.47)	0.48	71.3	20210.00
2	140.92	20.43	2.386	0.97( 0.47)	0.48	81.8	20200.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 141.24 Tc(MIN.) = 16.92  
 EFFECTIVE AREA(ACRES) = 71.34 AREA-AVERAGED Fm(INCH/HR) = 0.47  
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.48  
 TOTAL AREA(ACRES) = 81.8  
 LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20214.00 = 3451.11 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20214.00 TO NODE 20215.00 IS CODE = 62  
 -----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 13 USED)<<<<<  
 =====

UPSTREAM ELEVATION(FEET) = 1840.00 DOWNSTREAM ELEVATION(FEET) = 1793.00  
 STREET LENGTH(FEET) = 1205.58 CURB HEIGHT(INCHES) = 8.0  
 STREET HALFWIDTH(FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 176.89  
 \*\*\*STREET FLOWING FULL\*\*\*  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.82  
 HALFSTREET FLOOD WIDTH(FEET) = 39.52  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 7.62  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 6.23  
 STREET FLOW TRAVEL TIME(MIN.) = 2.64 Tc(MIN.) = 19.56  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.449  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	18.86	0.98	0.600	32
MOBILE HOME PARK	A	19.95	0.98	0.250	32
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98					
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.420					
SUBAREA AREA(ACRES) = 38.81		SUBAREA RUNOFF(CFS) = 71.25			
EFFECTIVE AREA(ACRES) = 110.15		AREA-AVERAGED Fm(INCH/HR) = 0.45			
AREA-AVERAGED Fp(INCH/HR) = 0.97		AREA-AVERAGED Ap = 0.46			
TOTAL AREA(ACRES) = 120.6		PEAK FLOW RATE(CFS) = 198.21			

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.84 HALFSTREET FLOOD WIDTH(FEET) = 40.68  
 FLOW VELOCITY(FEET/SEC.) = 7.91 DEPTH\*VELOCITY(FT\*FT/SEC.) = 6.65  
 \*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 1205.6 FT WITH ELEVATION-DROP = 47.0 FT, IS 106.6 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20215.00  
 LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20215.00 = 4656.69 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20215.00 TO NODE 20216.00 IS CODE = 62  
 -----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 13 USED)<<<<<  
 =====

UPSTREAM ELEVATION(FEET) = 1793.00 DOWNSTREAM ELEVATION(FEET) = 1740.00  
 STREET LENGTH(FEET) = 1725.28 CURB HEIGHT(INCHES) = 8.0  
 STREET HALFWIDTH(FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 237.31  
 \*\*\*STREET FLOWING FULL\*\*\*  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.91  
 HALFSTREET FLOOD WIDTH(FEET) = 44.04  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 7.72  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 7.00  
 STREET FLOW TRAVEL TIME(MIN.) = 3.73 Tc(MIN.) = 23.28  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.206  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	24.17	0.98	0.600	32
SCHOOL	A	9.62	0.98	0.600	32
MOBILE HOME PARK	A	14.92	0.98	0.250	32

COMMERCIAL A 0.89 0.98 0.100 32  
 RESIDENTIAL  
 "3-4 DWELLINGS/ACRE" B 0.13 0.75 0.600 56  
 COMMERCIAL B 0.31 0.75 0.100 56  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.484  
 SUBAREA AREA(ACRES) = 50.04 SUBAREA RUNOFF(CFS) = 78.13  
 EFFECTIVE AREA(ACRES) = 160.19 AREA-AVERAGED Fm(INCH/HR) = 0.46  
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.47  
 TOTAL AREA(ACRES) = 170.6 PEAK FLOW RATE(CFS) = 252.22

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.92 HALFSTREET FLOOD WIDTH(FEET) = 44.77  
 FLOW VELOCITY(FEET/SEC.) = 7.87 DEPTH\*VELOCITY(FT\*FT/SEC.) = 7.26  
 \*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 1725.3 FT WITH ELEVATION-DROP = 53.0 FT, IS 126.5 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20216.00  
 LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20216.00 = 6381.97 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20216.00 TO NODE 20232.00 IS CODE = 62

-----  
 >>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 13 USED)<<<<<

-----  
 UPSTREAM ELEVATION(FEET) = 1740.00 DOWNSTREAM ELEVATION(FEET) = 1739.00  
 STREET LENGTH(FEET) = 1052.00 CURB HEIGHT(INCHES) = 8.0  
 STREET HALFWIDTH(FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 260.11  
 \*\*\*STREET FLOWING FULL\*\*\*  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 1.61  
 HALFSTREET FLOOD WIDTH(FEET) = 79.07  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.22  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 3.57  
 STREET FLOW TRAVEL TIME(MIN.) = 7.89 Tc(MIN.) = 31.17  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.852  
 SUBAREA LOSS RATE DATA(AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
 MOBILE HOME PARK A 0.63 0.98 0.250 32  
 COMMERCIAL B 1.46 0.75 0.100 56  
 MOBILE HOME PARK B 4.91 0.75 0.250 56  
 RESIDENTIAL  
 "3-4 DWELLINGS/ACRE" B 4.10 0.75 0.600 56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.76  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.360  
 SUBAREA AREA(ACRES) = 11.10 SUBAREA RUNOFF(CFS) = 15.78  
 EFFECTIVE AREA(ACRES) = 171.29 AREA-AVERAGED Fm(INCH/HR) = 0.44  
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.46  
 TOTAL AREA(ACRES) = 181.7 PEAK FLOW RATE(CFS) = 252.22  
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 1.59 HALFSTREET FLOOD WIDTH(FEET) = 78.22  
 FLOW VELOCITY(FEET/SEC.) = 2.21 DEPTH\*VELOCITY(FT\*FT/SEC.) = 3.51  
 \*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 1052.0 FT WITH ELEVATION-DROP = 1.0 FT, IS 21.6 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20232.00  
 LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20232.00 = 7433.97 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20232.00 TO NODE 20232.00 IS CODE = 1

-----  
 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

-----  
 TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION(MIN.) = 31.17  
 RAINFALL INTENSITY(INCH/HR) = 1.85  
 AREA-AVERAGED Fm(INCH/HR) = 0.44  
 AREA-AVERAGED Fp(INCH/HR) = 0.96  
 AREA-AVERAGED Ap = 0.46  
 EFFECTIVE STREAM AREA(ACRES) = 171.29  
 TOTAL STREAM AREA(ACRES) = 181.70  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 252.22

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20220.00 TO NODE 20221.00 IS CODE = 21

-----  
 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

-----  
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 598.74  
 ELEVATION DATA: UPSTREAM(FEET) = 1935.00 DOWNSTREAM(FEET) = 1925.00

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20  
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.057  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.274  
 SUBAREA Tc AND LOSS RATE DATA(AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)  
 RESIDENTIAL  
 "3-4 DWELLINGS/ACRE" A 5.11 0.98 0.600 32 12.06  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.600  
 SUBAREA RUNOFF(CFS) = 12.37  
 TOTAL AREA(ACRES) = 5.11 PEAK FLOW RATE(CFS) = 12.37

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):



5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*

FLOW PROCESS FROM NODE 20221.00 TO NODE 20222.00 IS CODE = 92

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

UPSTREAM NODE ELEVATION (FEET) = 1925.00  
DOWNSTREAM NODE ELEVATION (FEET) = 1915.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 551.44  
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.250  
PAVEMENT LIP (FEET) = 0.100 MANNING'S N = .0150  
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.01700  
MAXIMUM DEPTH (FEET) = 1.00  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.978

SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN

RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 5.86 0.98 0.600 32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.98  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.600

TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 18.67  
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 4.46

AVERAGE FLOW DEPTH (FEET) = 0.57 FLOOD WIDTH (FEET) = 28.91  
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 2.06 Tc (MIN.) = 14.12

SUBAREA AREA (ACRES) = 5.86 SUBAREA RUNOFF (CFS) = 12.62  
EFFECTIVE AREA (ACRES) = 10.97 AREA-AVERAGED Fm (INCH/HR) = 0.59

AREA-AVERAGED Fp (INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.60  
TOTAL AREA (ACRES) = 11.0 PEAK FLOW RATE (CFS) = 23.63

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA "V" GUTTER HYDRAULICS:  
DEPTH (FEET) = 0.60 FLOOD WIDTH (FEET) = 32.65  
FLOW VELOCITY (FEET/SEC.) = 4.57 DEPTH\*VELOCITY (FT\*FT/SEC) = 2.75  
LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20222.00 = 1150.18 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20222.00 TO NODE 20223.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>(STREET TABLE SECTION # 5 USED)<<<<

=====

UPSTREAM ELEVATION (FEET) = 1915.00 DOWNSTREAM ELEVATION (FEET) = 1905.00  
STREET LENGTH (FEET) = 354.00 CURB HEIGHT (INCHES) = 6.0  
STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00  
INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 34.90

\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.51  
HALFSTREET FLOOD WIDTH (FEET) = 18.26  
AVERAGE FLOW VELOCITY (FEET/SEC.) = 4.72  
PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 2.38

STREET FLOW TRAVEL TIME (MIN.) = 1.25 Tc (MIN.) = 15.37

\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.830

SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN

RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 11.15 0.98 0.600 32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.98  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.600

SUBAREA AREA (ACRES) = 11.15 SUBAREA RUNOFF (CFS) = 22.53  
EFFECTIVE AREA (ACRES) = 22.12 AREA-AVERAGED Fm (INCH/HR) = 0.59

AREA-AVERAGED Fp (INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.60  
TOTAL AREA (ACRES) = 22.1 PEAK FLOW RATE (CFS) = 44.69

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH (FEET) = 0.54 HALFSTREET FLOOD WIDTH (FEET) = 19.84  
FLOW VELOCITY (FEET/SEC.) = 5.19 DEPTH\*VELOCITY (FT\*FT/SEC.) = 2.79  
LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20223.00 = 1504.18 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20223.00 TO NODE 20224.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>(STREET TABLE SECTION # 5 USED)<<<<

=====

UPSTREAM ELEVATION (FEET) = 1905.00 DOWNSTREAM ELEVATION (FEET) = 1895.00  
STREET LENGTH (FEET) = 253.00 CURB HEIGHT (INCHES) = 6.0  
STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00  
INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 52.32

\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 0.54  
HALFSTREET FLOOD WIDTH (FEET) = 19.78  
AVERAGE FLOW VELOCITY (FEET/SEC.) = 6.11  
PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 3.27

STREET FLOW TRAVEL TIME (MIN.) = 0.69 Tc (MIN.) = 16.06

\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.756

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
MOBILE HOME PARK	A	2.51	0.98	0.250	32
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	4.90	0.98	0.600	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.481  
 SUBAREA AREA(ACRES) = 7.41 SUBAREA RUNOFF(CFS) = 15.25  
 EFFECTIVE AREA(ACRES) = 29.53 AREA-AVERAGED Fm(INCH/HR) = 0.56  
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.57  
 TOTAL AREA(ACRES) = 29.5 PEAK FLOW RATE(CFS) = 58.48

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.55 HALFSTREET FLOOD WIDTH(FEET) = 20.58  
 FLOW VELOCITY(FEET/SEC.) = 6.36 DEPTH\*VELOCITY(FT\*FT/SEC.) = 3.51  
 LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20224.00 = 1757.18 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20224.00 TO NODE 20225.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 1895.00 DOWNSTREAM ELEVATION(FEET) = 1885.00  
 STREET LENGTH(FEET) = 323.50 CURB HEIGHT(INCHES) = 6.0  
 STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 68.27  
 \*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.60  
 HALFSTREET FLOOD WIDTH(FEET) = 22.77  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.15  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 3.66  
 STREET FLOW TRAVEL TIME(MIN.) = 0.88 Tc(MIN.) = 16.94  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.670

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
MOBILE HOME PARK	A	3.70	0.98	0.250	32
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	6.13	0.98	0.600	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.468  
 SUBAREA AREA(ACRES) = 9.83 SUBAREA RUNOFF(CFS) = 19.58  
 EFFECTIVE AREA(ACRES) = 39.36 AREA-AVERAGED Fm(INCH/HR) = 0.53

AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.54  
 TOTAL AREA(ACRES) = 39.4 PEAK FLOW RATE(CFS) = 75.76

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.61 HALFSTREET FLOOD WIDTH(FEET) = 23.69  
 FLOW VELOCITY(FEET/SEC.) = 6.34 DEPTH\*VELOCITY(FT\*FT/SEC.) = 3.89  
 \*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 323.5 FT WITH ELEVATION-DROP = 10.0 FT, IS 36.8 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20225.00  
 LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20225.00 = 2080.68 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20225.00 TO NODE 20226.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 1885.00 DOWNSTREAM ELEVATION(FEET) = 1875.00  
 STREET LENGTH(FEET) = 288.50 CURB HEIGHT(INCHES) = 6.0  
 STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 84.85  
 \*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.62  
 HALFSTREET FLOOD WIDTH(FEET) = 24.24  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.80  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 4.25  
 STREET FLOW TRAVEL TIME(MIN.) = 0.71 Tc(MIN.) = 17.64  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.605

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
MOBILE HOME PARK	A	6.40	0.98	0.250	32
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	2.52	0.98	0.600	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.349  
 SUBAREA AREA(ACRES) = 8.92 SUBAREA RUNOFF(CFS) = 18.18  
 EFFECTIVE AREA(ACRES) = 48.28 AREA-AVERAGED Fm(INCH/HR) = 0.50  
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.51  
 TOTAL AREA(ACRES) = 48.3 PEAK FLOW RATE(CFS) = 91.65

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.64 HALFSTREET FLOOD WIDTH(FEET) = 24.91  
 FLOW VELOCITY(FEET/SEC.) = 6.97 DEPTH\*VELOCITY(FT\*FT/SEC.) = 4.45  
 LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20226.00 = 2369.18 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20226.00 TO NODE 20227.00 IS CODE = 62  
 -----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<  
 =====

UPSTREAM ELEVATION(FEET) = 1875.00 DOWNSTREAM ELEVATION(FEET) = 1863.00  
 STREET LENGTH(FEET) = 404.50 CURB HEIGHT(INCHES) = 6.0  
 STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 104.21  
 \*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.68  
 HALFSTREET FLOOD WIDTH(FEET) = 26.98  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.81  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 4.63  
 STREET FLOW TRAVEL TIME(MIN.) = 0.99 Tc(MIN.) = 18.63  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.521

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
MOBILE HOME PARK	A	9.70	0.98	0.250	32
RESIDENTIAL "3-4 DWELLINGS/ACRE"	A	3.00	0.98	0.600	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.333  
 SUBAREA AREA(ACRES) = 12.70 SUBAREA RUNOFF(CFS) = 25.11  
 EFFECTIVE AREA(ACRES) = 60.98 AREA-AVERAGED Fm(INCH/HR) = 0.46  
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.47  
 TOTAL AREA(ACRES) = 61.0 PEAK FLOW RATE(CFS) = 113.12

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.70 HALFSTREET FLOOD WIDTH(FEET) = 27.84  
 FLOW VELOCITY(FEET/SEC.) = 6.97 DEPTH\*VELOCITY(FT\*FT/SEC.) = 4.86  
 \*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 404.5 FT WITH ELEVATION-DROP = 12.0 FT, IS 46.1 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20227.00  
 LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20227.00 = 2773.68 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20227.00 TO NODE 20228.00 IS CODE = 62  
 -----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<  
 =====

UPSTREAM ELEVATION(FEET) = 1863.00 DOWNSTREAM ELEVATION(FEET) = 1848.00  
 STREET LENGTH(FEET) = 374.50 CURB HEIGHT(INCHES) = 6.0  
 STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 122.87  
 \*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.68  
 HALFSTREET FLOOD WIDTH(FEET) = 27.17  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 7.93  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 5.42  
 STREET FLOW TRAVEL TIME(MIN.) = 0.79 Tc(MIN.) = 19.42  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.459

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
MOBILE HOME PARK	A	4.46	0.98	0.250	32
PUBLIC PARK	A	4.98	0.98	0.850	32
RESIDENTIAL "3-4 DWELLINGS/ACRE"	A	1.96	0.98	0.600	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.572  
 SUBAREA AREA(ACRES) = 11.40 SUBAREA RUNOFF(CFS) = 19.51  
 EFFECTIVE AREA(ACRES) = 72.38 AREA-AVERAGED Fm(INCH/HR) = 0.48  
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.49  
 TOTAL AREA(ACRES) = 72.4 PEAK FLOW RATE(CFS) = 129.23

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.69 HALFSTREET FLOOD WIDTH(FEET) = 27.66  
 FLOW VELOCITY(FEET/SEC.) = 8.06 DEPTH\*VELOCITY(FT\*FT/SEC.) = 5.59  
 \*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 374.5 FT WITH ELEVATION-DROP = 15.0 FT, IS 41.5 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20228.00  
 LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20228.00 = 3148.18 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20228.00 TO NODE 20229.00 IS CODE = 62  
 -----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<  
 =====

UPSTREAM ELEVATION(FEET) = 1848.00 DOWNSTREAM ELEVATION(FEET) = 1826.00  
STREET LENGTH(FEET) = 510.53 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 142.27  
\*\*\*STREET FLOWING FULL\*\*\*  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 0.71  
HALFSTREET FLOOD WIDTH(FEET) = 28.33  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 8.48  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 5.99  
STREET FLOW TRAVEL TIME(MIN.) = 1.00 Tc(MIN.) = 20.42  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.386  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
MOBILE HOME PARK A 5.30 0.98 0.250 32  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 4.30 0.98 0.600 32  
PUBLIC PARK A 6.33 0.98 0.850 32  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.97  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.583  
SUBAREA AREA(ACRES) = 15.93 SUBAREA RUNOFF(CFS) = 26.06  
EFFECTIVE AREA(ACRES) = 88.31 AREA-AVERAGED Fm(INCH/HR) = 0.49  
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.50  
TOTAL AREA(ACRES) = 88.3 PEAK FLOW RATE(CFS) = 150.53

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.16

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 0.72 HALFSTREET FLOOD WIDTH(FEET) = 28.94  
FLOW VELOCITY(FEET/SEC.) = 8.61 DEPTH\*VELOCITY(FT\*FT/SEC.) = 6.19  
\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
AND L = 510.5 FT WITH ELEVATION-DROP = 22.0 FT, IS 53.6 CFS,  
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20229.00  
LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20229.00 = 3658.71 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20229.00 TO NODE 20230.00 IS CODE = 62  
-----  
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<  
=====

UPSTREAM ELEVATION(FEET) = 1826.00 DOWNSTREAM ELEVATION(FEET) = 1800.00  
STREET LENGTH(FEET) = 713.66 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00

INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 168.36  
\*\*\*STREET FLOWING FULL\*\*\*  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 0.77  
HALFSTREET FLOOD WIDTH(FEET) = 31.26  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 8.31  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 6.35  
STREET FLOW TRAVEL TIME(MIN.) = 1.43 Tc(MIN.) = 21.86  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.291  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
MOBILE HOME PARK A 11.14 0.98 0.250 32  
PUBLIC PARK A 6.85 0.98 0.850 32  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 3.99 0.98 0.600 32  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.501  
SUBAREA AREA(ACRES) = 21.98 SUBAREA RUNOFF(CFS) = 35.67  
EFFECTIVE AREA(ACRES) = 110.29 AREA-AVERAGED Fm(INCH/HR) = 0.49  
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.50  
TOTAL AREA(ACRES) = 110.3 PEAK FLOW RATE(CFS) = 178.64

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.16

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 0.78 HALFSTREET FLOOD WIDTH(FEET) = 31.99  
FLOW VELOCITY(FEET/SEC.) = 8.43 DEPTH\*VELOCITY(FT\*FT/SEC.) = 6.57  
\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
AND L = 713.7 FT WITH ELEVATION-DROP = 26.0 FT, IS 67.4 CFS,  
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20230.00  
LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20230.00 = 4372.37 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20230.00 TO NODE 20231.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<  
=====

UPSTREAM ELEVATION(FEET) = 1800.00 DOWNSTREAM ELEVATION(FEET) = 1769.00  
STREET LENGTH(FEET) = 900.35 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 202.74  
\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.82  
HALFSTREET FLOOD WIDTH(FEET) = 34.00  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 8.50  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 6.97  
STREET FLOW TRAVEL TIME(MIN.) = 1.77 Tc(MIN.) = 23.62  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.187

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
MOBILE HOME PARK	A	14.01	0.98	0.250	32
MOBILE HOME PARK	B	8.21	0.75	0.250	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	2.69	0.98	0.600	32
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	3.23	0.75	0.600	56

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.88  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.324  
SUBAREA AREA(ACRES) = 28.14 SUBAREA RUNOFF(CFS) = 48.21  
EFFECTIVE AREA(ACRES) = 138.43 AREA-AVERAGED Fm(INCH/HR) = 0.45  
AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.47  
TOTAL AREA(ACRES) = 138.4 PEAK FLOW RATE(CFS) = 216.49

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.84 HALFSTREET FLOOD WIDTH(FEET) = 34.92  
FLOW VELOCITY(FEET/SEC.) = 8.62 DEPTH\*VELOCITY(FT\*FT/SEC.) = 7.23

\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,

AND L = 900.3 FT WITH ELEVATION-DROP = 31.0 FT, IS 85.5 CFS,  
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20231.00  
LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20231.00 = 5272.72 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20231.00 TO NODE 20232.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>(STREET TABLE SECTION # 5 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 1769.00 DOWNSTREAM ELEVATION(FEET) = 1739.00  
STREET LENGTH(FEET) = 905.39 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00

INSIDE STREET CROSSFALL(DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL(DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 229.79

\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.86  
HALFSTREET FLOOD WIDTH(FEET) = 36.02  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 8.61  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 7.41  
STREET FLOW TRAVEL TIME(MIN.) = 1.75 Tc(MIN.) = 25.37  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.095

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
MOBILE HOME PARK	A	0.17	0.98	0.250	32
MOBILE HOME PARK	B	5.75	0.75	0.250	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	11.10	0.75	0.600	56

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.478  
SUBAREA AREA(ACRES) = 17.02 SUBAREA RUNOFF(CFS) = 26.60  
EFFECTIVE AREA(ACRES) = 155.45 AREA-AVERAGED Fm(INCH/HR) = 0.44  
AREA-AVERAGED Fp(INCH/HR) = 0.94 AREA-AVERAGED Ap = 0.47  
TOTAL AREA(ACRES) = 155.4 PEAK FLOW RATE(CFS) = 231.64

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.86 HALFSTREET FLOOD WIDTH(FEET) = 36.08  
FLOW VELOCITY(FEET/SEC.) = 8.65 DEPTH\*VELOCITY(FT\*FT/SEC.) = 7.46

\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,

AND L = 905.4 FT WITH ELEVATION-DROP = 30.0 FT, IS 50.2 CFS,  
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20232.00  
LONGEST FLOWPATH FROM NODE 20220.00 TO NODE 20232.00 = 6178.11 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20232.00 TO NODE 20232.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 25.37

RAINFALL INTENSITY(INCH/HR) = 2.09

AREA-AVERAGED Fm(INCH/HR) = 0.44

AREA-AVERAGED Fp(INCH/HR) = 0.94

AREA-AVERAGED Ap = 0.47

EFFECTIVE STREAM AREA(ACRES) = 155.45

TOTAL STREAM AREA(ACRES) = 155.45

PEAK FLOW RATE(CFS) AT CONFLUENCE = 231.64

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	252.22	31.17	1.852	0.96( 0.44)	0.46	171.3	20210.00
1	240.67	34.86	1.731	0.96( 0.45)	0.46	181.7	20200.00
2	231.64	25.37	2.095	0.94( 0.44)	0.47	155.4	20220.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	472.47	25.37	2.095	0.95( 0.44)	0.47	294.9	20220.00
2	449.84	31.17	1.852	0.95( 0.44)	0.46	326.7	20210.00
3	421.47	34.86	1.731	0.95( 0.44)	0.47	337.2	20200.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 472.47 Tc(MIN.) = 25.37  
EFFECTIVE AREA(ACRES) = 294.90 AREA-AVERAGED Fm(INCH/HR) = 0.44  
AREA-AVERAGED Fp(INCH/HR) = 0.95 AREA-AVERAGED Ap = 0.47  
TOTAL AREA(ACRES) = 337.2  
LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20232.00 = 7433.97 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20232.00 TO NODE 20249.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 13 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 1739.00 DOWNSTREAM ELEVATION(FEET) = 1735.00  
STREET LENGTH(FEET) = 1274.82 CURB HEIGHT(INCHES) = 8.0  
STREET HALFWIDTH(FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 484.25  
\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 1.62  
HALFSTREET FLOOD WIDTH(FEET) = 79.81  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.06  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 6.58  
STREET FLOW TRAVEL TIME(MIN.) = 5.24 Tc(MIN.) = 30.61  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.872  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 0.11 0.98 0.600 32  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 18.30 0.75 0.600 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.600  
SUBAREA AREA(ACRES) = 18.41 SUBAREA RUNOFF(CFS) = 23.57  
EFFECTIVE AREA(ACRES) = 313.31 AREA-AVERAGED Fm(INCH/HR) = 0.44  
AREA-AVERAGED Fp(INCH/HR) = 0.93 AREA-AVERAGED Ap = 0.47  
TOTAL AREA(ACRES) = 355.6 PEAK FLOW RATE(CFS) = 472.47

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.31; 30M = 0.64; 1HR = 0.85; 3HR = 1.39; 6HR = 1.90; 24HR = 4.00

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 1.61 HALFSTREET FLOOD WIDTH(FEET) = 79.07  
FLOW VELOCITY(FEET/SEC.) = 4.04 DEPTH\*VELOCITY(FT\*FT/SEC.) = 6.49  
\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
AND L = 1274.8 FT WITH ELEVATION-DROP = 4.0 FT, IS 29.6 CFS,  
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20249.00  
LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20249.00 = 8708.79 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20249.00 TO NODE 20249.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 30.61  
RAINFALL INTENSITY(INCH/HR) = 1.87  
AREA-AVERAGED Fm(INCH/HR) = 0.44  
AREA-AVERAGED Fp(INCH/HR) = 0.93  
AREA-AVERAGED Ap = 0.47  
EFFECTIVE STREAM AREA(ACRES) = 313.31  
TOTAL STREAM AREA(ACRES) = 355.56  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 472.47

\*\*\*\*\*

FLOW PROCESS FROM NODE 20240.00 TO NODE 20241.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 916.98  
ELEVATION DATA: UPSTREAM(FEET) = 1880.00 DOWNSTREAM(FEET) = 1855.00

Tc = K \* [(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20  
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.964  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.134  
SUBAREA Tc AND LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 4.79 0.98 0.600 32 12.96  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 3.77 0.75 0.600 56 12.96  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.88  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.600  
SUBAREA RUNOFF(CFS) = 20.10  
TOTAL AREA(ACRES) = 8.56 PEAK FLOW RATE(CFS) = 20.10

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*

FLOW PROCESS FROM NODE 20241.00 TO NODE 20242.00 IS CODE = 92

-----  
>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<  
-----

UPSTREAM NODE ELEVATION (FEET) = 1855.00  
DOWNSTREAM NODE ELEVATION (FEET) = 1848.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 207.39  
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.250  
PAVEMENT LIP (FEET) = 0.100 MANNING'S N = .0150  
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.01700  
MAXIMUM DEPTH (FEET) = 1.00  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.055  
SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 1.59 0.98 0.600 32  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 2.06 0.75 0.600 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.85  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.600  
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 24.29  
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 6.06  
AVERAGE FLOW DEPTH (FEET) = 0.56 FLOOD WIDTH (FEET) = 28.17  
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 0.57 Tc (MIN.) = 13.53  
SUBAREA AREA (ACRES) = 3.65 SUBAREA RUNOFF (CFS) = 8.36  
EFFECTIVE AREA (ACRES) = 12.21 AREA-AVERAGED Fm (INCH/HR) = 0.52  
AREA-AVERAGED Fp (INCH/HR) = 0.87 AREA-AVERAGED Ap = 0.60  
TOTAL AREA (ACRES) = 12.2 PEAK FLOW RATE (CFS) = 27.85

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA "V" GUTTER HYDRAULICS:  
DEPTH (FEET) = 0.58 FLOOD WIDTH (FEET) = 30.41  
FLOW VELOCITY (FEET/SEC.) = 6.10 DEPTH\*VELOCITY (FT\*FT/SEC) = 3.56  
LONGEST FLOWPATH FROM NODE 20240.00 TO NODE 20242.00 = 1124.37 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20242.00 TO NODE 20243.00 IS CODE = 92  
-----

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<  
-----

UPSTREAM NODE ELEVATION (FEET) = 1848.00  
DOWNSTREAM NODE ELEVATION (FEET) = 1840.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 276.91  
"V" GUTTER WIDTH (FEET) = 3.00 GUTTER HIKE (FEET) = 0.250  
PAVEMENT LIP (FEET) = 0.100 MANNING'S N = .0150  
PAVEMENT CROSSFALL (DECIMAL NOTATION) = 0.01700  
MAXIMUM DEPTH (FEET) = 1.00  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.953  
SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 2.48 0.98 0.600 32  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 3.59 0.75 0.600 56  
RESIDENTIAL

" .4 DWELLING/ACRE" B 0.59 0.75 0.900 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.83  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.627  
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 35.14  
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 5.91  
AVERAGE FLOW DEPTH (FEET) = 0.62 FLOOD WIDTH (FEET) = 35.34  
"V" GUTTER FLOW TRAVEL TIME (MIN.) = 0.78 Tc (MIN.) = 14.31  
SUBAREA AREA (ACRES) = 6.66 SUBAREA RUNOFF (CFS) = 14.59  
EFFECTIVE AREA (ACRES) = 18.87 AREA-AVERAGED Fm (INCH/HR) = 0.52  
AREA-AVERAGED Fp (INCH/HR) = 0.85 AREA-AVERAGED Ap = 0.61  
TOTAL AREA (ACRES) = 18.9 PEAK FLOW RATE (CFS) = 41.33

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA "V" GUTTER HYDRAULICS:  
DEPTH (FEET) = 0.65 FLOOD WIDTH (FEET) = 38.17  
FLOW VELOCITY (FEET/SEC.) = 6.05 DEPTH\*VELOCITY (FT\*FT/SEC) = 3.93  
LONGEST FLOWPATH FROM NODE 20240.00 TO NODE 20243.00 = 1401.28 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20243.00 TO NODE 20244.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<  
>>>>(STREET TABLE SECTION # 18 USED)<<<<  
-----

UPSTREAM ELEVATION (FEET) = 1840.00 DOWNSTREAM ELEVATION (FEET) = 1830.00  
STREET LENGTH (FEET) = 293.50 CURB HEIGHT (INCHES) = 8.0  
STREET HALFWIDTH (FEET) = 26.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 15.00  
INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 50.29  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH (FEET) = 0.58  
HALFSTREET FLOOD WIDTH (FEET) = 21.05  
AVERAGE FLOW VELOCITY (FEET/SEC.) = 5.44  
PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 3.15  
STREET FLOW TRAVEL TIME (MIN.) = 0.90 Tc (MIN.) = 15.21  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.848

SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 3.29 0.98 0.600 32  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 4.18 0.75 0.600 56  
RESIDENTIAL  
".4 DWELLING/ACRE" B 1.12 0.75 0.900 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.83  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.639

SUBAREA AREA (ACRES) = 8.59 SUBAREA RUNOFF (CFS) = 17.92  
EFFECTIVE AREA (ACRES) = 27.46 AREA-AVERAGED Fm (INCH/HR) = 0.52  
AREA-AVERAGED Fp (INCH/HR) = 0.85 AREA-AVERAGED Ap = 0.62  
TOTAL AREA (ACRES) = 27.5 PEAK FLOW RATE (CFS) = 57.45

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH (FEET) = 0.60 HALFSTREET FLOOD WIDTH (FEET) = 22.16  
FLOW VELOCITY (FEET/SEC.) = 5.63 DEPTH\*VELOCITY (FT\*FT/SEC.) = 3.39  
LONGEST FLOWPATH FROM NODE 20240.00 TO NODE 20244.00 = 1694.78 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20244.00 TO NODE 20245.00 IS CODE = 62  
-----

>>>> COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA <<<<<<  
>>>> (STREET TABLE SECTION # 18 USED) <<<<<<  
-----

UPSTREAM ELEVATION (FEET) = 1830.00 DOWNSTREAM ELEVATION (FEET) = 1815.00  
STREET LENGTH (FEET) = 273.00 CURB HEIGHT (INCHES) = 8.0  
STREET HALFWIDTH (FEET) = 26.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 15.00  
INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 65.28  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH (FEET) = 0.58  
HALFSTREET FLOOD WIDTH (FEET) = 21.22  
AVERAGE FLOW VELOCITY (FEET/SEC.) = 6.95  
PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 4.05  
STREET FLOW TRAVEL TIME (MIN.) = 0.65 Tc (MIN.) = 15.87  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.776  
SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 2.55 0.98 0.600 32  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 4.04 0.75 0.600 56  
RESIDENTIAL  
".4 DWELLING/ACRE" B 1.15 0.75 0.900 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.82  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.645  
SUBAREA AREA (ACRES) = 7.74 SUBAREA RUNOFF (CFS) = 15.67  
EFFECTIVE AREA (ACRES) = 35.20 AREA-AVERAGED Fm (INCH/HR) = 0.52  
AREA-AVERAGED Fp (INCH/HR) = 0.84 AREA-AVERAGED Ap = 0.62  
TOTAL AREA (ACRES) = 35.2 PEAK FLOW RATE (CFS) = 71.36

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH (FEET) = 0.60 HALFSTREET FLOOD WIDTH (FEET) = 21.99  
FLOW VELOCITY (FEET/SEC.) = 7.10 DEPTH\*VELOCITY (FT\*FT/SEC.) = 4.25  
LONGEST FLOWPATH FROM NODE 20240.00 TO NODE 20245.00 = 1967.78 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20245.00 TO NODE 20246.00 IS CODE = 62  
-----

>>>> COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA <<<<<<  
>>>> (STREET TABLE SECTION # 18 USED) <<<<<<  
-----

UPSTREAM ELEVATION (FEET) = 1815.00 DOWNSTREAM ELEVATION (FEET) = 1805.00  
STREET LENGTH (FEET) = 359.00 CURB HEIGHT (INCHES) = 8.0  
STREET HALFWIDTH (FEET) = 26.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 15.00  
INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 81.22  
\*\*\*STREET FLOWING FULL\*\*\*  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH (FEET) = 0.68  
HALFSTREET FLOOD WIDTH (FEET) = 26.91  
AVERAGE FLOW VELOCITY (FEET/SEC.) = 5.69  
PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 3.90  
STREET FLOW TRAVEL TIME (MIN.) = 1.05 Tc (MIN.) = 16.92  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.672

SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" A 3.90 0.98 0.600 32  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 5.36 0.75 0.600 56  
RESIDENTIAL  
".4 DWELLING/ACRE" B 0.93 0.75 0.900 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.83  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.627  
SUBAREA AREA (ACRES) = 10.19 SUBAREA RUNOFF (CFS) = 19.72  
EFFECTIVE AREA (ACRES) = 45.39 AREA-AVERAGED Fm (INCH/HR) = 0.52  
AREA-AVERAGED Fp (INCH/HR) = 0.84 AREA-AVERAGED Ap = 0.63  
TOTAL AREA (ACRES) = 45.4 PEAK FLOW RATE (CFS) = 87.76

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.81

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH (FEET) = 0.70 HALFSTREET FLOOD WIDTH (FEET) = 27.52  
FLOW VELOCITY (FEET/SEC.) = 5.88 DEPTH\*VELOCITY (FT\*FT/SEC.) = 4.10  
LONGEST FLOWPATH FROM NODE 20240.00 TO NODE 20246.00 = 2326.78 FEET.



```

*****
FLOW PROCESS FROM NODE 20246.00 TO NODE 20247.00 IS CODE = 62
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STREET TABLE SECTION # 18 USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 1805.00 DOWNSTREAM ELEVATION(FEET) = 1795.00
STREET LENGTH(FEET) = 324.04 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 26.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 15.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 95.67
***STREET FLOWING FULL***
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.70
HALFSTREET FLOOD WIDTH(FEET) = 27.83
AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.27
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 4.41
STREET FLOW TRAVEL TIME(MIN.) = 0.86 Tc(MIN.) = 17.78
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.593
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"3-4 DWELLINGS/ACRE" A 3.02 0.98 0.600 32
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B 4.88 0.75 0.600 56
RESIDENTIAL
".4 DWELLING/ACRE" B 0.55 0.75 0.900 56
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.83
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.620
SUBAREA AREA(ACRES) = 8.45 SUBAREA RUNOFF(CFS) = 15.83
EFFECTIVE AREA(ACRES) = 53.84 AREA-AVERAGED Fm(INCH/HR) = 0.52
AREA-AVERAGED Fp(INCH/HR) = 0.84 AREA-AVERAGED Ap = 0.62
TOTAL AREA(ACRES) = 53.8 PEAK FLOW RATE(CFS) = 100.38

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.71 HALFSTREET FLOOD WIDTH(FEET) = 28.26
FLOW VELOCITY(FEET/SEC.) = 6.38 DEPTH*VELOCITY(FT*FT/SEC.) = 4.54
LONGEST FLOWPATH FROM NODE 20240.00 TO NODE 20247.00 = 2650.82 FEET.

```

```

*****
FLOW PROCESS FROM NODE 20247.00 TO NODE 20248.00 IS CODE = 62
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STREET TABLE SECTION # 18 USED)<<<<<
=====

```

```

UPSTREAM ELEVATION(FEET) = 1795.00 DOWNSTREAM ELEVATION(FEET) = 1782.00
STREET LENGTH(FEET) = 263.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 26.00

```

```

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 15.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

```

```

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

```

```

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 107.22
***STREET FLOWING FULL***
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.68
HALFSTREET FLOOD WIDTH(FEET) = 26.85
AVERAGE FLOW VELOCITY(FEET/SEC.) = 7.55
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 5.16
STREET FLOW TRAVEL TIME(MIN.) = 0.58 Tc(MIN.) = 18.36
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.544

```

```

SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"3-4 DWELLINGS/ACRE" A 1.94 0.98 0.600 32
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B 5.00 0.75 0.600 56
RESIDENTIAL
".4 DWELLING/ACRE" B 0.49 0.75 0.900 56
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.81
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.620
SUBAREA AREA(ACRES) = 7.43 SUBAREA RUNOFF(CFS) = 13.67
EFFECTIVE AREA(ACRES) = 61.27 AREA-AVERAGED Fm(INCH/HR) = 0.52
AREA-AVERAGED Fp(INCH/HR) = 0.83 AREA-AVERAGED Ap = 0.62
TOTAL AREA(ACRES) = 61.3 PEAK FLOW RATE(CFS) = 111.66

```

```

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

```

```

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.69 HALFSTREET FLOOD WIDTH(FEET) = 27.16
FLOW VELOCITY(FEET/SEC.) = 7.69 DEPTH*VELOCITY(FT*FT/SEC.) = 5.30
LONGEST FLOWPATH FROM NODE 20240.00 TO NODE 20248.00 = 2913.82 FEET.

```

```

*****
FLOW PROCESS FROM NODE 20248.00 TO NODE 20249.00 IS CODE = 62
-----

```

```

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STREET TABLE SECTION # 18 USED)<<<<<
=====

```

```

UPSTREAM ELEVATION(FEET) = 1782.00 DOWNSTREAM ELEVATION(FEET) = 1735.00
STREET LENGTH(FEET) = 1589.51 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 26.00

```

```

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 15.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020

```

OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL(DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 129.76

\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.77

HALFSTREET FLOOD WIDTH(FEET) = 30.94

AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.86

PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 5.25

STREET FLOW TRAVEL TIME(MIN.) = 3.86 Tc(MIN.) = 22.22

\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.268

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	A	0.28	0.98	0.600	32
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	21.09	0.75	0.600	56
RESIDENTIAL					
".4 DWELLING/ACRE"	B	0.85	0.75	0.900	56

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75

SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.611

SUBAREA AREA(ACRES) = 22.22 SUBAREA RUNOFF(CFS) = 36.18

EFFECTIVE AREA(ACRES) = 83.49 AREA-AVERAGED Fm(INCH/HR) = 0.50

AREA-AVERAGED Fp(INCH/HR) = 0.81 AREA-AVERAGED Ap = 0.62

TOTAL AREA(ACRES) = 83.5 PEAK FLOW RATE(CFS) = 132.65

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.77 HALFSTREET FLOOD WIDTH(FEET) = 31.19

FLOW VELOCITY(FEET/SEC.) = 6.90 DEPTH\*VELOCITY(FT\*FT/SEC.) = 5.31

LONGEST FLOWPATH FROM NODE 20240.00 TO NODE 20249.00 = 4503.33 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20249.00 TO NODE 20249.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 22.22

RAINFALL INTENSITY(INCH/HR) = 2.27

AREA-AVERAGED Fm(INCH/HR) = 0.50

AREA-AVERAGED Fp(INCH/HR) = 0.81

AREA-AVERAGED Ap = 0.62

EFFECTIVE STREAM AREA(ACRES) = 83.49

TOTAL STREAM AREA(ACRES) = 83.49

PEAK FLOW RATE(CFS) AT CONFLUENCE = 132.65

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	472.47	30.61	1.872	0.93( 0.44)	0.47	313.3	20220.00
1	449.84	36.47	1.685	0.94( 0.44)	0.47	345.2	20210.00
1	421.47	40.24	1.588	0.94( 0.44)	0.47	355.6	20200.00
2	132.65	22.22	2.268	0.81( 0.50)	0.62	83.5	20240.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	570.79	22.22	2.268	0.89( 0.46)	0.51	311.0	20240.00
2	575.33	30.61	1.872	0.90( 0.45)	0.50	396.8	20220.00
3	538.68	36.47	1.685	0.91( 0.45)	0.50	428.6	20210.00
4	503.03	40.24	1.588	0.91( 0.45)	0.50	439.1	20200.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 575.33 Tc(MIN.) = 30.61

EFFECTIVE AREA(ACRES) = 396.80 AREA-AVERAGED Fm(INCH/HR) = 0.45

AREA-AVERAGED Fp(INCH/HR) = 0.90 AREA-AVERAGED Ap = 0.50

TOTAL AREA(ACRES) = 439.1

LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20249.00 = 8708.79 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20249.00 TO NODE 20250.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>(STREET TABLE SECTION # 13 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 1735.00 DOWNSTREAM ELEVATION(FEET) = 1733.00

STREET LENGTH(FEET) = 391.69 CURB HEIGHT(INCHES) = 8.0

STREET HALFWIDTH(FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00

INSIDE STREET CROSSFALL(DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL(DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 575.90

\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 1.58

HALFSTREET FLOOD WIDTH(FEET) = 77.85

AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.09

PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 8.06

STREET FLOW TRAVEL TIME(MIN.) = 1.28 Tc(MIN.) = 31.89

\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.826

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.58	0.75	0.600	56

RESIDENTIAL  
".4 DWELLING/ACRE" B 0.42 0.75 0.900 56  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.726  
SUBAREA AREA(ACRES) = 1.00 SUBAREA RUNOFF(CFS) = 1.15  
EFFECTIVE AREA(ACRES) = 397.80 AREA-AVERAGED Fm(INCH/HR) = 0.46  
AREA-AVERAGED Fp(INCH/HR) = 0.90 AREA-AVERAGED Ap = 0.50  
TOTAL AREA(ACRES) = 440.1 PEAK FLOW RATE(CFS) = 575.33  
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH( FEET) = 1.58 HALFSTREET FLOOD WIDTH( FEET) = 77.79  
FLOW VELOCITY( FEET/SEC.) = 5.09 DEPTH\*VELOCITY( FT\*FT/SEC.) = 8.06  
LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20250.00 = 9100.48 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20250.00 TO NODE 20250.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<<

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20151.00 TO NODE 20151.00 IS CODE = 15.1

>>>>DEFINE MEMORY BANK # 2 <<<<<<

PEAK FLOWRATE TABLE FILE NAME: 20151.DNA  
MEMORY BANK # 2 DEFINED AS FOLLOWS:  
PEAK FLOW RATE(CFS) = 1868.80 Tc(MIN.) = 36.88  
AREA-AVERAGED Fm(INCH/HR) = 0.61 Ybar = 0.53  
TOTAL AREA(ACRES) = 1725.0  
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20151.00 = 15438.18 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20151.00 TO NODE 20151.00 IS CODE = 14.0

>>>>MEMORY BANK # 2 COPIED ONTO MAIN-STREAM MEMORY<<<<<<

MAIN-STREAM MEMORY DEFINED AS FOLLOWS:  
PEAK FLOW RATE(CFS) = 1868.80 Tc(MIN.) = 36.88  
AREA-AVERAGED Fm(INCH/HR) = 0.61 Ybar = 0.53  
TOTAL AREA(ACRES) = 1725.0  
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20151.00 = 15438.18 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20151.00 TO NODE 20151.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 2 <<<<<<

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20151.00 TO NODE 20250.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<<

ELEVATION DATA: UPSTREAM( FEET) = 1785.00 DOWNSTREAM( FEET) = 1733.00  
CHANNEL LENGTH THRU SUBAREA( FEET) = 1656.68 CHANNEL SLOPE = 0.0314  
CHANNEL BASE( FEET) = 10.00 "Z" FACTOR = 2.000  
MANNING'S FACTOR = 0.035 MAXIMUM DEPTH( FEET) = 10.00  
CHANNEL FLOW THRU SUBAREA( CFS) = 1868.80  
FLOW VELOCITY( FEET/SEC.) = 16.64 FLOW DEPTH( FEET) = 5.40  
TRAVEL TIME( MIN.) = 1.66 Tc( MIN.) = 38.54  
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20250.00 = 17094.86 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20250.00 TO NODE 20250.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<<

\*\*\*\*\*  
MAINLINE Tc( MIN.) = 38.54  
\* 100 YEAR RAINFALL INTENSITY( INCH/HR) = 1.630  
SUBAREA LOSS RATE DATA( AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	1.58	0.75	0.600	56
RESIDENTIAL					
".4 DWELLING/ACRE"	B	54.48	0.75	0.900	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.892  
SUBAREA AREA(ACRES) = 56.06  
UNIT-HYDROGRAPH DATA:  
RAINFALL( INCH): 5M= 0.46;30M= 0.95;1H= 1.25;3H= 2.18;6H= 3.09;24H= 7.48  
S-GRAPH: VALLEY( DEV.) = 23.5%;VALLEY( UNDEV.) /DESERT( UNDEV.) = 76.5%  
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT( UNDEV.) = 0.0%

Tc( HR) = 0.64; LAG( HR) = 0.51; Fm( INCH/HR) = 0.61; Ybar = 0.54  
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.  
DEPTH-AREA FACTORS: 5M = 0.92; 30M = 0.92; 1HR = 0.92;  
3HR = 0.99; 6HR = 0.99; 24HR= 1.00  
UNIT-INTERVAL( MIN) = 5.00 TOTAL AREA( ACRES) = 1781.1  
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20250.00 = 17094.86 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:  
Lca/L=0.3,n=.0440; Lca/L=0.4,n=.0394; Lca/L=0.5,n=.0362;Lca/L=0.6,n=.0338  
TIME OF PEAK FLOW( HR) = 16.50 RUNOFF VOLUME( AF) = 543.57  
UNIT-HYDROGRAPH PEAK FLOW RATE( CFS) = 1643.88  
TOTAL AREA( ACRES) = 1781.1 PEAK FLOW RATE( CFS) = 1868.80  
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH( INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.71

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20250.00 TO NODE 20250.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<<

\*\*\*\*\*  
\*\* MAIN STREAM CONFLUENCE DATA \*\*  
PEAK FLOW RATE( CFS) = 1868.80 Tc( MIN.) = 38.54  
AREA-AVERAGED Fm( INCH/HR) = 0.61 Ybar = 0.54  
TOTAL AREA( ACRES) = 1781.1

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20250.00 = 17094.86 FEET.

\*\* MEMORY BANK # 1 CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	570.79	23.51	2.193	0.89( 0.46)	0.51	312.0	20240.00
2	575.33	31.89	1.826	0.90( 0.46)	0.50	397.8	20220.00
3	538.68	37.77	1.650	0.91( 0.45)	0.50	429.6	20210.00
4	503.03	41.57	1.558	0.91( 0.45)	0.50	440.1	20200.00

LONGEST FLOWPATH FROM NODE 20200.00 TO NODE 20250.00 = 9100.48 FEET.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

UNIT-HYDROGRAPH DATA:

RAINFALL(INCH): 5M= 0.46;30M= 0.95;1H= 1.25;3H= 2.14;6H= 3.02;24H= 7.36

S-GRAPH: VALLEY (DEV.)= 38.4%;VALLEY (UNDEV.)/DESERT= 61.6%

MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT (UNDEV.)= 0.0%

Tc (HR) = 0.64; LAG (HR) = 0.51; Fm (INCH/HR) = 0.58; Ybar = 0.52

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.90; 30M = 0.90; 1HR = 0.90;

3HR = 0.99; 6HR = 0.99; 24HR = 1.00

UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 2221.2

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20250.00 = 17094.86 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3,n=.0440; Lca/L=0.4,n=.0394; Lca/L=0.5,n=.0362;Lca/L=0.6,n=.0338

TIME OF PEAK FLOW (HR) = 16.50 RUNOFF VOLUME (AF) = 683.26

PEAK FLOW RATE (CFS) = 2050.54

\*\*\*\*\*

FLOW PROCESS FROM NODE 20250.00 TO NODE 20250.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

\*\*\*\*\*

FLOW PROCESS FROM NODE 20250.00 TO NODE 20274.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 1733.00 DOWNSTREAM (FEET) = 1670.00

CHANNEL LENGTH THRU SUBAREA (FEET) = 2379.03 CHANNEL SLOPE = 0.0265

CHANNEL BASE (FEET) = 10.00 "Z" FACTOR = 2.000

MANNING'S FACTOR = 0.035 MAXIMUM DEPTH (FEET) = 10.00

CHANNEL FLOW THRU SUBAREA (CFS) = 2050.54

FLOW VELOCITY (FEET/SEC.) = 16.00 FLOW DEPTH (FEET) = 5.89

TRAVEL TIME (MIN.) = 2.48 Tc (MIN.) = 41.02

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20274.00 = 19473.89 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20274.00 TO NODE 20274.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 41.02

\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.570

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN

RESIDENTIAL

"3-4 DWELLINGS/ACRE" B 3.23 0.75 0.600 56

RESIDENTIAL

"3-4 DWELLINGS/ACRE" A 0.07 0.98 0.600 32

RESIDENTIAL

".4 DWELLING/ACRE" B 9.49 0.75 0.900 56

SCHOOL B 24.91 0.75 0.600 56

SCHOOL A 0.90 0.98 0.600 32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.75

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.674

SUBAREA AREA (ACRES) = 38.60

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M= 0.46;30M= 0.95;1H= 1.25;3H= 2.14;6H= 3.01;24H= 7.35

S-GRAPH: VALLEY (DEV.)= 39.0%;VALLEY (UNDEV.)/DESERT= 61.0%

MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT (UNDEV.)= 0.0%

Tc (HR) = 0.68; LAG (HR) = 0.55; Fm (INCH/HR) = 0.58; Ybar = 0.52

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.90; 30M = 0.90; 1HR = 0.90;

3HR = 0.98; 6HR = 0.99; 24HR = 1.00

UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 2259.8

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20274.00 = 19473.89 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3,n=.0417; Lca/L=0.4,n=.0374; Lca/L=0.5,n=.0343;Lca/L=0.6,n=.0320

TIME OF PEAK FLOW (HR) = 16.58 RUNOFF VOLUME (AF) = 694.27

UNIT-HYDROGRAPH PEAK FLOW RATE (CFS) = 2005.21

TOTAL AREA (ACRES) = 2259.8 PEAK FLOW RATE (CFS) = 2050.54

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*

FLOW PROCESS FROM NODE 20274.00 TO NODE 20274.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

PEAK FLOW RATE (CFS) = 2050.54 Tc (MIN.) = 41.02

AREA-AVERAGED Fm (INCH/HR) = 0.58 Ybar = 0.52

TOTAL AREA (ACRES) = 2259.8

\*\*\*\*\*

FLOW PROCESS FROM NODE 20260.00 TO NODE 20261.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 680.83

ELEVATION DATA: UPSTREAM (FEET) = 2600.00 DOWNSTREAM (FEET) = 2360.00

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 7.333

\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.412

SUBAREA Tc AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
NATURAL FAIR COVER						

"OPEN BRUSH" B 4.43 0.61 1.000 66 11.82  
 RESIDENTIAL  
 "2 DWELLINGS/ACRE" B 2.14 0.75 0.700 56 7.33  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.65  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.902  
 SUBAREA RUNOFF(CFS) = 22.63  
 TOTAL AREA (ACRES) = 6.57 PEAK FLOW RATE (CFS) = 22.63

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20261.00 TO NODE 20262.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 2360.00 DOWNSTREAM(FEET) = 2280.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 583.76 CHANNEL SLOPE = 0.1370  
 CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 1.00  
 CHANNEL FLOW THRU SUBAREA(CFS) = 22.63  
 FLOW VELOCITY(FEET/SEC.) = 3.78 FLOW DEPTH(FEET) = 0.35  
 TRAVEL TIME(MIN.) = 2.58 Tc(MIN.) = 9.91  
 LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20262.00 = 1264.59 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20262.00 TO NODE 20262.00 IS CODE = 81  
 -----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 9.91  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.683  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	4.44	0.75	0.700	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	15.90	0.61	1.000	66

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.64  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.935  
 SUBAREA AREA(ACRES) = 20.34 SUBAREA RUNOFF(CFS) = 56.54  
 EFFECTIVE AREA(ACRES) = 26.91 AREA-AVERAGED Fm(INCH/HR) = 0.59  
 AREA-AVERAGED Fp(INCH/HR) = 0.64 AREA-AVERAGED Ap = 0.93  
 TOTAL AREA(ACRES) = 26.9 PEAK FLOW RATE(CFS) = 74.86

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20262.00 TO NODE 20263.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 2280.00 DOWNSTREAM(FEET) = 2170.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 994.37 CHANNEL SLOPE = 0.1106

CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
 CHANNEL FLOW THRU SUBAREA(CFS) = 74.86  
 FLOW VELOCITY(FEET/SEC.) = 4.75 FLOW DEPTH(FEET) = 0.56  
 TRAVEL TIME(MIN.) = 3.49 Tc(MIN.) = 13.40  
 LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20263.00 = 2258.96 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20263.00 TO NODE 20263.00 IS CODE = 81  
 -----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 13.40  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.073  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	8.82	0.75	0.700	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	15.90	0.61	1.000	66

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.700  
 SUBAREA AREA(ACRES) = 8.82 SUBAREA RUNOFF(CFS) = 20.23  
 EFFECTIVE AREA(ACRES) = 35.73 AREA-AVERAGED Fm(INCH/HR) = 0.58  
 AREA-AVERAGED Fp(INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.87  
 TOTAL AREA(ACRES) = 35.7 PEAK FLOW RATE(CFS) = 80.32

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20263.00 TO NODE 20264.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 2170.00 DOWNSTREAM(FEET) = 2110.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 784.49 CHANNEL SLOPE = 0.0765  
 CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
 CHANNEL FLOW THRU SUBAREA(CFS) = 80.32  
 FLOW VELOCITY(FEET/SEC.) = 4.17 FLOW DEPTH(FEET) = 0.62  
 TRAVEL TIME(MIN.) = 3.14 Tc(MIN.) = 16.54  
 LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20264.00 = 3043.45 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20264.00 TO NODE 20264.00 IS CODE = 81  
 -----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 16.54  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.708  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	17.48	0.75	0.700	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	7.48	0.61	1.000	66

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.70  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.790  
SUBAREA AREA(ACRES) = 24.96 SUBAREA RUNOFF(CFS) = 48.47  
EFFECTIVE AREA(ACRES) = 60.69 AREA-AVERAGED Fm(INCH/HR) = 0.57  
AREA-AVERAGED Fp(INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.84  
TOTAL AREA(ACRES) = 60.7 PEAK FLOW RATE(CFS) = 117.07

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20264.00 TO NODE 20265.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	2110.00	DOWNSTREAM(FEET) =	2080.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	660.96	CHANNEL SLOPE =	0.0454
CHANNEL BASE(FEET) =	0.00	"Z" FACTOR =	50.000
MANNING'S FACTOR =	0.045	MAXIMUM DEPTH(FEET) =	3.00
CHANNEL FLOW THRU SUBAREA(CFS) =	117.07		
FLOW VELOCITY(FEET/SEC.) =	3.76	FLOW DEPTH(FEET) =	0.79
TRAVEL TIME(MIN.) =	2.93	Tc(MIN.) =	19.46
LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20265.00 =	3704.41	FEET.	

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20265.00 TO NODE 20265.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) =	19.46				
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	2.456				
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	6.85	0.75	0.700	56
RESIDENTIAL					
".4 DWELLING/ACRE"	B	0.71	0.75	0.900	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	59.45	0.61	1.000	66
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =	0.63				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	0.968				
SUBAREA AREA(ACRES) =	67.01	SUBAREA RUNOFF(CFS) =	111.62		
EFFECTIVE AREA(ACRES) =	127.70	AREA-AVERAGED Fm(INCH/HR) =	0.59		
AREA-AVERAGED Fp(INCH/HR) =	0.65	AREA-AVERAGED Ap =	0.91		
TOTAL AREA(ACRES) =	127.7	PEAK FLOW RATE(CFS) =	214.91		

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20265.00 TO NODE 20266.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	2080.00	DOWNSTREAM(FEET) =	2010.00
----------------------------------	---------	--------------------	---------

CHANNEL LENGTH THRU SUBAREA(FEET) = 947.22 CHANNEL SLOPE = 0.0739  
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 3.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 214.91  
FLOW VELOCITY(FEET/SEC.) = 5.27 FLOW DEPTH(FEET) = 0.90  
TRAVEL TIME(MIN.) = 3.00 Tc(MIN.) = 22.46  
LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20266.00 = 4651.63 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20266.00 TO NODE 20266.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) =	22.46				
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	2.254				
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	10.89	0.75	0.700	56
RESIDENTIAL					
".4 DWELLING/ACRE"	B	11.99	0.75	0.900	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	4.30	0.61	1.000	66
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =	0.72				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	0.836				
SUBAREA AREA(ACRES) =	27.18	SUBAREA RUNOFF(CFS) =	40.36		
EFFECTIVE AREA(ACRES) =	154.88	AREA-AVERAGED Fm(INCH/HR) =	0.59		
AREA-AVERAGED Fp(INCH/HR) =	0.66	AREA-AVERAGED Ap =	0.89		
TOTAL AREA(ACRES) =	154.9	PEAK FLOW RATE(CFS) =	232.04		

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20266.00 TO NODE 20267.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	2010.00	DOWNSTREAM(FEET) =	1960.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	906.98	CHANNEL SLOPE =	0.0551
CHANNEL BASE(FEET) =	0.00	"Z" FACTOR =	50.000
MANNING'S FACTOR =	0.045	MAXIMUM DEPTH(FEET) =	3.00
CHANNEL FLOW THRU SUBAREA(CFS) =	232.04		
FLOW VELOCITY(FEET/SEC.) =	4.82	FLOW DEPTH(FEET) =	0.98
TRAVEL TIME(MIN.) =	3.14	Tc(MIN.) =	25.60
LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20267.00 =	5558.61	FEET.	

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20267.00 TO NODE 20267.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) =	25.60				
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	2.084				
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS

LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	53.81	0.75	0.700	56
RESIDENTIAL					
".4 DWELLING/ACRE"	B	46.51	0.75	0.900	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	68.77	0.61	1.000	66

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.69  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.877  
 SUBAREA AREA (ACRES) = 169.09      SUBAREA RUNOFF (CFS) = 225.60  
 EFFECTIVE AREA (ACRES) = 323.97      AREA-AVERAGED Fm (INCH/HR) = 0.60  
 AREA-AVERAGED Fp (INCH/HR) = 0.67      AREA-AVERAGED Ap = 0.89  
 TOTAL AREA (ACRES) = 324.0      PEAK FLOW RATE (CFS) = 433.93

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20267.00 TO NODE 20268.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 1960.00      DOWNSTREAM (FEET) = 1890.00  
 CHANNEL LENGTH THRU SUBAREA (FEET) = 1268.00      CHANNEL SLOPE = 0.0552  
 CHANNEL BASE (FEET) = 10.00      "Z" FACTOR = 2.000  
 MANNING'S FACTOR = 0.045      MAXIMUM DEPTH (FEET) = 5.00  
 CHANNEL FLOW THRU SUBAREA (CFS) = 433.93  
 FLOW VELOCITY (FEET/SEC.) = 11.43      FLOW DEPTH (FEET) = 2.52  
 TRAVEL TIME (MIN.) = 1.85      Tc (MIN.) = 27.45  
 LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20268.00 = 6826.61 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20268.00 TO NODE 20268.00 IS CODE = 81  
 -----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
".4 DWELLING/ACRE"	B	30.11	0.75	0.900	56
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	0.46	0.75	0.700	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.75  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.897  
 SUBAREA AREA (ACRES) = 30.57      SUBAREA RUNOFF (CFS) = 36.53  
 EFFECTIVE AREA (ACRES) = 354.54      AREA-AVERAGED Fm (INCH/HR) = 0.60  
 AREA-AVERAGED Fp (INCH/HR) = 0.68      AREA-AVERAGED Ap = 0.89  
 TOTAL AREA (ACRES) = 354.5      PEAK FLOW RATE (CFS) = 445.57

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20268.00 TO NODE 20269.00 IS CODE = 54  
 -----

-----  
 >>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 1890.00      DOWNSTREAM (FEET) = 1870.00  
 CHANNEL LENGTH THRU SUBAREA (FEET) = 379.58      CHANNEL SLOPE = 0.0527  
 CHANNEL BASE (FEET) = 10.00      "Z" FACTOR = 2.000  
 MANNING'S FACTOR = 0.045      MAXIMUM DEPTH (FEET) = 5.00  
 CHANNEL FLOW THRU SUBAREA (CFS) = 445.57  
 FLOW VELOCITY (FEET/SEC.) = 11.33      FLOW DEPTH (FEET) = 2.59  
 TRAVEL TIME (MIN.) = 0.56      Tc (MIN.) = 28.00  
 LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20269.00 = 7206.19 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20269.00 TO NODE 20269.00 IS CODE = 81  
 -----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
".4 DWELLING/ACRE"	B	17.99	0.75	0.900	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.04	0.75	0.600	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	18.04	0.61	1.000	66
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	16.31	0.75	0.700	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.70  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.872  
 SUBAREA AREA (ACRES) = 52.38      SUBAREA RUNOFF (CFS) = 64.51  
 EFFECTIVE AREA (ACRES) = 406.92      AREA-AVERAGED Fm (INCH/HR) = 0.60  
 AREA-AVERAGED Fp (INCH/HR) = 0.68      AREA-AVERAGED Ap = 0.88  
 TOTAL AREA (ACRES) = 406.9      PEAK FLOW RATE (CFS) = 502.42

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20269.00 TO NODE 20270.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM (FEET) = 1870.00      DOWNSTREAM (FEET) = 1770.00  
 CHANNEL LENGTH THRU SUBAREA (FEET) = 2346.89      CHANNEL SLOPE = 0.0426  
 CHANNEL BASE (FEET) = 10.00      "Z" FACTOR = 2.000  
 MANNING'S FACTOR = 0.045      MAXIMUM DEPTH (FEET) = 5.00  
 CHANNEL FLOW THRU SUBAREA (CFS) = 502.42  
 FLOW VELOCITY (FEET/SEC.) = 10.84      FLOW DEPTH (FEET) = 2.92  
 TRAVEL TIME (MIN.) = 3.61      Tc (MIN.) = 31.61  
 LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20270.00 = 9553.08 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20270.00 TO NODE 20270.00 IS CODE = 81  
 -----

=====  
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<  
=====

MAINLINE Tc(MIN.) = 31.61  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.836  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 5.45 0.75 0.600 56  
RESIDENTIAL  
".4 DWELLING/ACRE" B 71.00 0.75 0.900 56  
NATURAL FAIR COVER  
"OPEN BRUSH" B 5.28 0.61 1.000 66  
RESIDENTIAL  
"2 DWELLINGS/ACRE" B 40.34 0.75 0.700 56  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.825  
SUBAREA AREA(ACRES) = 122.07 SUBAREA RUNOFF(CFS) = 134.56  
EFFECTIVE AREA(ACRES) = 528.99 AREA-AVERAGED Fm(INCH/HR) = 0.60  
AREA-AVERAGED Fp(INCH/HR) = 0.69 AREA-AVERAGED Ap = 0.87  
TOTAL AREA(ACRES) = 529.0 PEAK FLOW RATE(CFS) = 586.25

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.31; 30M = 0.64; 1HR = 0.85; 3HR = 1.39; 6HR = 1.90; 24HR = 4.17

\*\*\*\*\*

FLOW PROCESS FROM NODE 20270.00 TO NODE 20271.00 IS CODE = 62

=====  
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<  
>>>>(STREET TABLE SECTION # 13 USED)<<<<  
=====

UPSTREAM ELEVATION(FEET) = 1770.00 DOWNSTREAM ELEVATION(FEET) = 1755.00  
STREET LENGTH(FEET) = 692.85 CURB HEIGHT(INCHES) = 8.0  
STREET HALFWIDTH(FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 667.58  
\*\*\*STREET FLOWING FULL\*\*\*  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 1.30  
HALFSTREET FLOOD WIDTH(FEET) = 63.88  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 9.07  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 11.84  
STREET FLOW TRAVEL TIME(MIN.) = 1.27 Tc(MIN.) = 32.89  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.793  
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL

" .4 DWELLING/ACRE" B 100.00 0.75 0.900 56  
RESIDENTIAL  
".4 DWELLING/ACRE" B 27.18 0.75 0.900 56  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 11.00 0.75 0.600 56  
RESIDENTIAL  
"2 DWELLINGS/ACRE" B 18.36 0.75 0.700 56  
NATURAL FAIR COVER  
"OPEN BRUSH" B 0.17 0.61 1.000 66  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.856  
SUBAREA AREA(ACRES) = 156.71 SUBAREA RUNOFF(CFS) = 162.65  
EFFECTIVE AREA(ACRES) = 685.70 AREA-AVERAGED Fm(INCH/HR) = 0.61  
AREA-AVERAGED Fp(INCH/HR) = 0.71 AREA-AVERAGED Ap = 0.87  
TOTAL AREA(ACRES) = 685.7 PEAK FLOW RATE(CFS) = 728.45

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.31; 30M = 0.64; 1HR = 0.85; 3HR = 1.40; 6HR = 1.91; 24HR = 4.34

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 1.34 HALFSTREET FLOOD WIDTH(FEET) = 65.77  
FLOW VELOCITY(FEET/SEC.) = 9.28 DEPTH\*VELOCITY(FT\*FT/SEC.) = 12.46  
\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
AND L = 692.8 FT WITH ELEVATION-DROP = 15.0 FT, IS 369.7 CFS,  
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20271.00  
LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20271.00 = 10245.93 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 20270.00 TO NODE 20271.00 IS CODE = 71

=====  
>>>>PEAK FLOW RATE ESTIMATOR CHANGED TO UNIT-HYDROGRAPH METHOD<<<<  
>>>>USING TIME-OF-CONCENTRATION OF LONGEST FLOWPATH<<<<  
=====

UNIT-HYDROGRAPH DATA:  
RAINFALL(INCH): 5M= 0.40;30M= 0.82;1H= 1.09;3H= 1.77;6H= 2.41;24H= 6.18  
S-GRAPH: VALLEY(DEV.) = 28.6%;VALLEY(UNDEV.)/DESERT= 71.4%  
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.) = 0.0%  
Tc(HR) = 0.55; LAG(HR) = 0.44; Fm(INCH/HR) = 0.61; Ybar = 0.60  
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.  
DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;  
3HR = 1.00; 6HR = 1.00; 24HR= 1.00  
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 685.7  
LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20271.00 = 10245.93 FEET.  
EQUIVALENT BASIN FACTOR APPROXIMATIONS:  
Lca/L=0.3,n=.0553; Lca/L=0.4,n=.0496; Lca/L=0.5,n=.0456;Lca/L=0.6,n=.0425  
TIME OF PEAK FLOW(HR) = 16.42 RUNOFF VOLUME(AF) = 154.43  
UNIT-HYDROGRAPH METHOD PEAK FLOW RATE(CFS) = 606.38  
TOTAL PEAK FLOW RATE(CFS) = 606.38 (SOURCE FLOW INCLUDED)  
RATIONAL METHOD PEAK FLOW RATE(CFS) = 728.45  
(UPSTREAM NODE PEAK FLOW RATE(CFS) = 728.45)  
PEAK FLOW RATE(CFS) USED = 728.45

\*\*\*\*\*

FLOW PROCESS FROM NODE 20271.00 TO NODE 20272.00 IS CODE = 62

=====  
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<  
>>>>(STREET TABLE SECTION # 13 USED)<<<<  
=====



UPSTREAM ELEVATION (FEET) = 1755.00 DOWNSTREAM ELEVATION (FEET) = 1730.00  
STREET LENGTH (FEET) = 1359.40 CURB HEIGHT (INCHES) = 8.0  
STREET HALFWIDTH (FEET) = 32.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 20.00  
INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 774.87  
\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 1.41  
HALFSTREET FLOOD WIDTH (FEET) = 69.06  
AVERAGE FLOW VELOCITY (FEET/SEC.) = 8.87  
PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 12.49  
STREET FLOW TRAVEL TIME (MIN.) = 2.55 Tc (MIN.) = 35.44  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.714

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL ".4 DWELLING/ACRE"	B	92.29	0.75	0.900	56
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	5.58	0.75	0.600	56

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.75

SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.883

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M= 0.41; 30M= 0.84; 1H= 1.11; 3H= 1.80; 6H= 2.45; 24H= 6.28

S-GRAPH: VALLEY (DEV.) = 25.8%; VALLEY (UNDEV.) / DESERT = 74.2%

MOUNTAIN = 0.0%; FOOTHILL = 0.0%; DESERT (UNDEV.) = 0.0%

Tc (HR) = 0.59; LAG (HR) = 0.47; Fm (INCH/HR) = 0.62; Ybar = 0.60

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;

3HR = 0.99; 6HR = 1.00; 24HR = 1.00

UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 783.6

LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20272.00 = 11605.33 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3, n=.0533; Lca/L=0.4, n=.0478; Lca/L=0.5, n=.0439; Lca/L=0.6, n=.0409

TIME OF PEAK FLOW (HR) = 16.50 RUNOFF VOLUME (AF) = 178.37

UNIT-HYDROGRAPH PEAK FLOW RATE (CFS) = 667.31

TOTAL AREA (ACRES) = 783.6 PEAK FLOW RATE (CFS) = 728.45

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.91

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 1.38 HALFSTREET FLOOD WIDTH (FEET) = 67.66

FLOW VELOCITY (FEET/SEC.) = 8.72 DEPTH\*VELOCITY (FT\*FT/SEC.) = 12.04

\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,

AND L = 1359.4 FT WITH ELEVATION-DROP = 25.0 FT, IS 181.4 CFS,  
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20272.00

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20272.00 TO NODE 20273.00 IS CODE = 62  
-----

>>>> COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA <<<<<<  
>>>> (STREET TABLE SECTION # 5 USED) <<<<<<  
-----

UPSTREAM ELEVATION (FEET) = 1730.00 DOWNSTREAM ELEVATION (FEET) = 1695.00  
STREET LENGTH (FEET) = 1247.53 CURB HEIGHT (INCHES) = 6.0  
STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00  
INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 753.41

\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 1.33  
HALFSTREET FLOOD WIDTH (FEET) = 59.58  
AVERAGE FLOW VELOCITY (FEET/SEC.) = 10.50  
PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 13.99  
STREET FLOW TRAVEL TIME (MIN.) = 1.98 Tc (MIN.) = 37.42  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.659

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	2.91	0.75	0.600	56
RESIDENTIAL ".4 DWELLING/ACRE"	B	52.68	0.75	0.900	56

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.75

SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.884

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M= 0.41; 30M= 0.85; 1H= 1.12; 3H= 1.82; 6H= 2.47; 24H= 6.29

S-GRAPH: VALLEY (DEV.) = 24.4%; VALLEY (UNDEV.) / DESERT = 75.6%

MOUNTAIN = 0.0%; FOOTHILL = 0.0%; DESERT (UNDEV.) = 0.0%

Tc (HR) = 0.62; LAG (HR) = 0.50; Fm (INCH/HR) = 0.62; Ybar = 0.60

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;

3HR = 0.99; 6HR = 1.00; 24HR = 1.00

UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 839.2

LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20273.00 = 12852.86 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3, n=.0514; Lca/L=0.4, n=.0461; Lca/L=0.5, n=.0424; Lca/L=0.6, n=.0395

TIME OF PEAK FLOW (HR) = 16.50 RUNOFF VOLUME (AF) = 190.65

UNIT-HYDROGRAPH PEAK FLOW RATE (CFS) = 694.76

TOTAL AREA (ACRES) = 839.2 PEAK FLOW RATE (CFS) = 728.45

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 1.32 HALFSTREET FLOOD WIDTH (FEET) = 58.78  
FLOW VELOCITY (FEET/SEC.) = 10.43 DEPTH\*VELOCITY (FT\*FT/SEC.) = 13.72  
\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
AND L = 1247.5 FT WITH ELEVATION-DROP = 35.0 FT, IS 113.1 CFS,  
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20273.00

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20273.00 TO NODE 20274.00 IS CODE = 62

>>>> COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA <<<<<<  
>>>> (STREET TABLE SECTION # 5 USED) <<<<<<

=====

UPSTREAM ELEVATION (FEET) = 1695.00 DOWNSTREAM ELEVATION (FEET) = 1670.00  
STREET LENGTH (FEET) = 797.55 CURB HEIGHT (INCHES) = 6.0  
STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00  
INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 729.84  
\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH (FEET) = 1.29  
HALFSTREET FLOOD WIDTH (FEET) = 57.56  
AVERAGE FLOW VELOCITY (FEET/SEC.) = 10.89  
PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 14.07  
STREET FLOW TRAVEL TIME (MIN.) = 1.22 Tc (MIN.) = 38.64  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.628

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
".4 DWELLING/ACRE"	B	2.08	0.75	0.900	56
SCHOOL	B	0.94	0.75	0.600	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.807

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M = 0.41; 30M = 0.85; 1H = 1.12; 3H = 1.82; 6H = 2.47; 24H = 6.29  
S-GRAPH: VALLEY (DEV.) = 24.4%; VALLEY (UNDEV.) / DESERT = 75.6%

MOUNTAIN = 0.0%; FOOTHILL = 0.0%; DESERT (UNDEV.) = 0.0%  
Tc (HR) = 0.64; LAG (HR) = 0.52; Fm (INCH/HR) = 0.62; Ybar = 0.60

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;

3HR = 0.99; 6HR = 1.00; 24HR = 1.00

UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 842.2

LONGEST FLOWPATH FROM NODE 20260.00 TO NODE 20274.00 = 13650.41 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3, n=.0504; Lca/L=0.4, n=.0452; Lca/L=0.5, n=.0415; Lca/L=0.6, n=.0387

TIME OF PEAK FLOW (HR) = 16.50 RUNOFF VOLUME (AF) = 191.40

UNIT-HYDROGRAPH PEAK FLOW RATE (CFS) = 681.73

TOTAL AREA (ACRES) = 842.2 PEAK FLOW RATE (CFS) = 728.45

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 1.29 HALFSTREET FLOOD WIDTH (FEET) = 57.50

FLOW VELOCITY (FEET/SEC.) = 10.89 DEPTH\*VELOCITY (FT\*FT/SEC.) = 14.05

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20274.00 TO NODE 20274.00 IS CODE = 1

>>>> DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE <<<<<<

>>>> AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES <<<<<<

=====

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

PEAK FLOW RATE (CFS) = 728.45 Tc (MIN.) = 38.64

AREA-AVERAGED Fm (INCH/HR) = 0.62 Ybar = 0.60

TOTAL AREA (ACRES) = 842.2

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	AREA (ACRES)	HEADWATER NODE
1	2050.54	41.02	2259.75	20120.00
2	728.45	38.64	842.18	20260.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M = 0.45; 30M = 0.92; 1H = 1.21; 3H = 2.06; 6H = 2.87; 24H = 7.06

S-GRAPH: VALLEY (DEV.) = 35.0%; VALLEY (UNDEV.) / DESERT = 65.0%

MOUNTAIN = 0.0%; FOOTHILL = 0.0%; DESERT (UNDEV.) = 0.0%

Tc (HR) = 0.68; LAG (HR) = 0.55; Fm (INCH/HR) = 0.59; Ybar = 0.54

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.86; 30M = 0.86; 1HR = 0.86;

3HR = 0.98; 6HR = 0.99; 24HR = 0.99

UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 3101.9

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20274.00 = 19473.89 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3, n=.0417; Lca/L=0.4, n=.0374; Lca/L=0.5, n=.0343; Lca/L=0.6, n=.0320

TIME OF PEAK FLOW (HR) = 16.58 RUNOFF VOLUME (AF) = 876.23

PEAK FLOW RATE (CFS) = 2478.03

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20274.00 TO NODE 20274.00 IS CODE = 152

>>>> STORE PEAK FLOWRATE TABLE TO A FILE <<<<<<

=====

PEAK FLOWRATE TABLE FILE NAME: 20274.DNA

=====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 3101.9 TC (MIN.) = 41.02

AREA-AVERAGED Fm (INCH/HR) = 0.59 Ybar = 0.54

PEAK FLOW RATE (CFS) = 2478.03

=====

END OF INTEGRATED RATIONAL/UNIT-HYDROGRAPH METHOD ANALYSIS



\*\*\*\*\*  
RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
(Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)  
(c) Copyright 1983-2013 Advanced Engineering Software (aes)  
Ver. 20.0 Release Date: 06/01/2013 License ID 1264

Analysis prepared by:

\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* REDLANDS MPD - UPDATE \*  
\* REVISED RATIONAL METHOD HYDROLOGY - TO NODE 20353 \*  
\* BY TMLI SEPT 2013 \*  
\*\*\*\*\*

FILE NAME: LR0203.DAT  
TIME/DATE OF STUDY: 15:32 09/24/2013

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--\*TIME-OF-CONCENTRATION MODEL\*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00  
SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00  
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.85  
\*USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL\*

SLOPE OF INTENSITY DURATION CURVE(LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000  
USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.2340

\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD\*

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF- CROWN TO	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES:			MANNING FACTOR (n)	
	WIDTH (FT)			CROSSFALL (FT)	WIDTH (FT)	LIP (FT)		HIKE (FT)
1	18.0	12.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
2	20.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
3	22.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
4	15.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
5	18.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
6	15.0	10.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
7	16.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
8	16.0	10.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
9	17.0	10.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
10	30.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
11	24.0	15.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
12	24.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
13	32.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
14	39.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
15	36.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
16	12.5	5.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180

17	20.0	10.0	0.020/0.020/0.020	0.50	1.50	0.0312	0.125	0.0180
18	26.0	15.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180
19	52.0	20.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0180

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.20 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
  2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)
- \*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*  
\*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

UNIT-HYDROGRAPH MODEL SELECTIONS/PARAMETERS:

WATERSHED LAG = 0.80 \* Tc  
USED "VALLEY UNDEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 1 UNITS/ACRE AND LESS; AND "VALLEY DEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 2 UNITS/ACRE AND MORE.  
PRECIPITATION DATA ENTERED ON SUBAREA BASIS.  
SIERRA MADRE DEPTH-AREA FACTORS USED.  
\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR UNIT HYDROGRAPH METHOD\*

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20300.00 TO NODE 20301.00 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 658.37  
ELEVATION DATA: UPSTREAM(FEET) = 2600.00 DOWNSTREAM(FEET) = 2400.00

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20  
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.287  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.047  
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
NATURAL FAIR COVER						
"OPEN BRUSH"	B	6.22	0.61	1.000	66	12.01
RESIDENTIAL						
".4 DWELLING/ACRE"	B	0.99	0.75	0.900	56	8.29

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.63  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.986  
SUBAREA RUNOFF(CFS) = 22.23  
TOTAL AREA(ACRES) = 7.21 PEAK FLOW RATE(CFS) = 22.23

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20301.00 TO NODE 20302.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 2400.00 DOWNSTREAM(FEET) = 2380.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 422.45 CHANNEL SLOPE = 0.0473  
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000

MANNING'S FACTOR = 0.045 MAXIMUM DEPTH (FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA (CFS) = 22.23  
FLOW VELOCITY (FEET/SEC.) = 2.56 FLOW DEPTH (FEET) = 0.42  
TRAVEL TIME (MIN.) = 2.75 Tc (MIN.) = 11.04  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20302.00 = 1080.82 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20302.00 TO NODE 20302.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

MAINLINE Tc (MIN.) = 11.04  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.408  
SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
".4 DWELLING/ACRE" B 0.12 0.75 0.900 56  
NATURAL FAIR COVER  
"OPEN BRUSH" B 4.14 0.61 1.000 66  
SCHOOL B 3.66 0.75 0.600 56  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.66  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.814  
SUBAREA AREA (ACRES) = 7.92 SUBAREA RUNOFF (CFS) = 20.45  
EFFECTIVE AREA (ACRES) = 15.13 AREA-AVERAGED Fm (INCH/HR) = 0.58  
AREA-AVERAGED Fp (INCH/HR) = 0.65 AREA-AVERAGED Ap = 0.90  
TOTAL AREA (ACRES) = 15.1 PEAK FLOW RATE (CFS) = 38.53

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20302.00 TO NODE 20303.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM (FEET) = 2380.00 DOWNSTREAM (FEET) = 2320.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 870.68 CHANNEL SLOPE = 0.0689  
CHANNEL BASE (FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH (FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA (CFS) = 38.53  
FLOW VELOCITY (FEET/SEC.) = 3.38 FLOW DEPTH (FEET) = 0.48  
TRAVEL TIME (MIN.) = 4.30 Tc (MIN.) = 15.34  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20303.00 = 1951.50 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20303.00 TO NODE 20303.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

MAINLINE Tc (MIN.) = 15.34  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.797  
SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
NATURAL FAIR COVER  
"OPEN BRUSH" B 4.15 0.61 1.000 66

RESIDENTIAL  
".4 DWELLING/ACRE" B 0.80 0.75 0.900 56  
SCHOOL B 20.38 0.75 0.600 56  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.72  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.675  
SUBAREA AREA (ACRES) = 25.33 SUBAREA RUNOFF (CFS) = 52.76  
EFFECTIVE AREA (ACRES) = 40.46 AREA-AVERAGED Fm (INCH/HR) = 0.52  
AREA-AVERAGED Fp (INCH/HR) = 0.68 AREA-AVERAGED Ap = 0.76  
TOTAL AREA (ACRES) = 40.5 PEAK FLOW RATE (CFS) = 82.98

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20303.00 TO NODE 20304.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM (FEET) = 2320.00 DOWNSTREAM (FEET) = 2280.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 981.07 CHANNEL SLOPE = 0.0408  
CHANNEL BASE (FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH (FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA (CFS) = 82.98  
FLOW VELOCITY (FEET/SEC.) = 3.32 FLOW DEPTH (FEET) = 0.71  
TRAVEL TIME (MIN.) = 4.92 Tc (MIN.) = 20.26  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20304.00 = 2932.57 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20304.00 TO NODE 20304.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

MAINLINE Tc (MIN.) = 20.26  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.367  
SUBAREA LOSS RATE DATA (AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
NATURAL FAIR COVER  
"OPEN BRUSH" B 18.37 0.61 1.000 66  
SCHOOL B 15.66 0.75 0.600 56  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.66  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.816  
SUBAREA AREA (ACRES) = 34.03 SUBAREA RUNOFF (CFS) = 56.03  
EFFECTIVE AREA (ACRES) = 74.49 AREA-AVERAGED Fm (INCH/HR) = 0.53  
AREA-AVERAGED Fp (INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.78  
TOTAL AREA (ACRES) = 74.5 PEAK FLOW RATE (CFS) = 123.34

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20304.00 TO NODE 20305.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

ELEVATION DATA: UPSTREAM (FEET) = 2280.00 DOWNSTREAM (FEET) = 2220.00

CHANNEL LENGTH THRU SUBAREA (FEET) = 823.37 CHANNEL SLOPE = 0.0729  
CHANNEL BASE (FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH (FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA (CFS) = 123.34  
FLOW VELOCITY (FEET/SEC.) = 4.56 FLOW DEPTH (FEET) = 0.74  
TRAVEL TIME (MIN.) = 3.01 Tc (MIN.) = 23.27  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20305.00 = 3755.94 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20305.00 TO NODE 20305.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc (MIN.) = 23.27  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 2.179  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER "OPEN BRUSH"	B	9.94	0.61	1.000	66
RESIDENTIAL "2 DWELLINGS/ACRE"	B	0.01	0.75	0.700	56
SCHOOL	B	7.91	0.75	0.600	56

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.66  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.823  
SUBAREA AREA (ACRES) = 17.86 SUBAREA RUNOFF (CFS) = 26.33  
EFFECTIVE AREA (ACRES) = 92.35 AREA-AVERAGED Fm (INCH/HR) = 0.53  
AREA-AVERAGED Fp (INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.79  
TOTAL AREA (ACRES) = 92.3 PEAK FLOW RATE (CFS) = 137.02

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20305.00 TO NODE 20306.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 2220.00 DOWNSTREAM (FEET) = 2190.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 801.97 CHANNEL SLOPE = 0.0374  
CHANNEL BASE (FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH (FEET) = 3.00  
CHANNEL FLOW THRU SUBAREA (CFS) = 137.02  
FLOW VELOCITY (FEET/SEC.) = 3.66 FLOW DEPTH (FEET) = 0.86  
TRAVEL TIME (MIN.) = 3.65 Tc (MIN.) = 26.91  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20306.00 = 4557.91 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20306.00 TO NODE 20306.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc (MIN.) = 26.91  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.996  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER "OPEN BRUSH"	B	9.94	0.61	1.000	66
RESIDENTIAL "2 DWELLINGS/ACRE"	B	0.01	0.75	0.700	56
SCHOOL	B	7.91	0.75	0.600	56

RESIDENTIAL  
"2 DWELLINGS/ACRE" B 1.66 0.75 0.700 56  
NATURAL FAIR COVER  
"OPEN BRUSH" B 13.33 0.61 1.000 66  
SCHOOL B 2.17 0.75 0.600 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.63  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.920  
SUBAREA AREA (ACRES) = 17.16 SUBAREA RUNOFF (CFS) = 21.81  
EFFECTIVE AREA (ACRES) = 109.51 AREA-AVERAGED Fm (INCH/HR) = 0.54  
AREA-AVERAGED Fp (INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.81  
TOTAL AREA (ACRES) = 109.5 PEAK FLOW RATE (CFS) = 143.67

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20306.00 TO NODE 20307.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 2190.00 DOWNSTREAM (FEET) = 2185.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 181.13 CHANNEL SLOPE = 0.0276  
CHANNEL BASE (FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH (FEET) = 3.00  
CHANNEL FLOW THRU SUBAREA (CFS) = 143.67  
FLOW VELOCITY (FEET/SEC.) = 3.30 FLOW DEPTH (FEET) = 0.93  
TRAVEL TIME (MIN.) = 0.92 Tc (MIN.) = 27.83  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20307.00 = 4739.04 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20307.00 TO NODE 20307.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc (MIN.) = 27.83  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.957  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "2 DWELLINGS/ACRE"	B	1.33	0.75	0.700	56
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	0.26	0.75	0.600	56
NATURAL FAIR COVER "OPEN BRUSH"	B	3.26	0.61	1.000	66

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp (INCH/HR) = 0.65  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.896  
SUBAREA AREA (ACRES) = 4.85 SUBAREA RUNOFF (CFS) = 6.01  
EFFECTIVE AREA (ACRES) = 114.36 AREA-AVERAGED Fm (INCH/HR) = 0.54  
AREA-AVERAGED Fp (INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.82  
TOTAL AREA (ACRES) = 114.4 PEAK FLOW RATE (CFS) = 145.77

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20307.00 TO NODE 20308.00 IS CODE = 54  
-----

```

-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 2185.00 DOWNSTREAM(FEET) = 2175.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 269.83 CHANNEL SLOPE = 0.0371
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 4.00
CHANNEL FLOW THRU SUBAREA(CFS) = 145.77
FLOW VELOCITY(FEET/SEC.) = 3.70 FLOW DEPTH(FEET) = 0.89
TRAVEL TIME(MIN.) = 1.22 Tc(MIN.) = 29.05
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20308.00 = 5008.87 FEET.

```

```

*****
FLOW PROCESS FROM NODE 20308.00 TO NODE 20308.00 IS CODE = 81
-----

```

```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 29.05
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.907
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA      Fp      Ap      SCS
LAND USE           GROUP   (ACRES)  (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"2 DWELLINGS/ACRE"   B       2.10     0.75    0.700    56
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B       0.65     0.75    0.600    56
NATURAL FAIR COVER
"OPEN BRUSH"         B       1.26     0.61    1.000    66
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.69
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.778
SUBAREA AREA(ACRES) = 4.01 SUBAREA RUNOFF(CFS) = 4.93
EFFECTIVE AREA(ACRES) = 118.37 AREA-AVERAGED Fm(INCH/HR) = 0.54
AREA-AVERAGED Fp(INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.81
TOTAL AREA(ACRES) = 118.4 PEAK FLOW RATE(CFS) = 145.77
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

```

```

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

```

```

*****
FLOW PROCESS FROM NODE 20308.00 TO NODE 20309.00 IS CODE = 62
-----

```

```

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STREET TABLE SECTION # 5 USED)<<<<
=====

```

```

UPSTREAM ELEVATION(FEET) = 2175.00 DOWNSTREAM ELEVATION(FEET) = 2150.00
STREET LENGTH(FEET) = 430.92 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

```

```

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

```

```

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

```

```

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 149.09
***STREET FLOWING FULL***
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.68
HALFSTREET FLOOD WIDTH(FEET) = 27.23
AVERAGE FLOW VELOCITY(FEET/SEC.) = 9.58
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 6.56
STREET FLOW TRAVEL TIME(MIN.) = 0.75 Tc(MIN.) = 29.80
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.878
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA      Fp      Ap      SCS
LAND USE           GROUP   (ACRES)  (INCH/HR) (DECIMAL) CN
NATURAL FAIR COVER
"OPEN BRUSH"         B       1.71     0.61    1.000    66
RESIDENTIAL
"2 DWELLINGS/ACRE"   B       2.80     0.75    0.700    56
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B       1.00     0.75    0.600    56
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.69
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.775
SUBAREA AREA(ACRES) = 5.51 SUBAREA RUNOFF(CFS) = 6.65
EFFECTIVE AREA(ACRES) = 123.88 AREA-AVERAGED Fm(INCH/HR) = 0.54
AREA-AVERAGED Fp(INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.81
TOTAL AREA(ACRES) = 123.9 PEAK FLOW RATE(CFS) = 149.16

```

```

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

```

```

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.68 HALFSTREET FLOOD WIDTH(FEET) = 27.23
FLOW VELOCITY(FEET/SEC.) = 9.59 DEPTH*VELOCITY(FT*FT/SEC.) = 6.56
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20309.00 = 5439.79 FEET.

```

```

*****
FLOW PROCESS FROM NODE 20309.00 TO NODE 20310.00 IS CODE = 62
-----

```

```

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STREET TABLE SECTION # 5 USED)<<<<
=====

```

```

UPSTREAM ELEVATION(FEET) = 2150.00 DOWNSTREAM ELEVATION(FEET) = 2140.00
STREET LENGTH(FEET) = 330.10 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 18.00

```

```

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

```

```

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

```

```

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 152.34
***STREET FLOWING FULL***
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.76
HALFSTREET FLOOD WIDTH(FEET) = 31.13

```

AVERAGE FLOW VELOCITY (FEET/SEC.) = 7.57  
 PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 5.77  
 STREET FLOW TRAVEL TIME (MIN.) = 0.73 Tc (MIN.) = 30.52  
 \* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.851  
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	3.69	0.75	0.700	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	0.85	0.61	1.000	66
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.79	0.75	0.600	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.72  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.733  
 SUBAREA AREA (ACRES) = 5.33 SUBAREA RUNOFF (CFS) = 6.35  
 EFFECTIVE AREA (ACRES) = 129.21 AREA-AVERAGED Fm (INCH/HR) = 0.54  
 AREA-AVERAGED Fp (INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.81  
 TOTAL AREA (ACRES) = 129.2 PEAK FLOW RATE (CFS) = 152.51

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH (FEET) = 0.76 HALFSTREET FLOOD WIDTH (FEET) = 31.20  
 FLOW VELOCITY (FEET/SEC.) = 7.55 DEPTH\*VELOCITY (FT\*FT/SEC.) = 5.77  
 LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20310.00 = 5769.89 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20310.00 TO NODE 20311.00 IS CODE = 62  
 -----

>>>> COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA <<<<<<  
 >>>> (STREET TABLE SECTION # 5 USED) <<<<<<  
 =====

UPSTREAM ELEVATION (FEET) = 2140.00 DOWNSTREAM ELEVATION (FEET) = 2100.00  
 STREET LENGTH (FEET) = 329.50 CURB HEIGHT (INCHES) = 6.0  
 STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00  
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 155.51  
 \*\*\*STREET FLOWING FULL\*\*\*  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH (FEET) = 0.62  
 HALFSTREET FLOOD WIDTH (FEET) = 24.05  
 AVERAGE FLOW VELOCITY (FEET/SEC.) = 12.64  
 PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 7.85  
 STREET FLOW TRAVEL TIME (MIN.) = 0.43 Tc (MIN.) = 30.96  
 \* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.835  
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	4.27	0.75	0.700	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	5.25	0.61	1.000	66
RESIDENTIAL					

LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	2.87	0.75	0.700	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	1.50	0.61	1.000	66
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.78	0.75	0.600	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.70  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.772  
 SUBAREA AREA (ACRES) = 5.15 SUBAREA RUNOFF (CFS) = 6.01  
 EFFECTIVE AREA (ACRES) = 134.36 AREA-AVERAGED Fm (INCH/HR) = 0.54  
 AREA-AVERAGED Fp (INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.81  
 TOTAL AREA (ACRES) = 134.4 PEAK FLOW RATE (CFS) = 156.70

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50  
 END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH (FEET) = 0.62 HALFSTREET FLOOD WIDTH (FEET) = 24.12  
 FLOW VELOCITY (FEET/SEC.) = 12.67 DEPTH\*VELOCITY (FT\*FT/SEC.) = 7.89  
 LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20311.00 = 6099.39 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20311.00 TO NODE 20312.00 IS CODE = 62  
 -----

>>>> COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA <<<<<<  
 >>>> (STREET TABLE SECTION # 5 USED) <<<<<<  
 =====

UPSTREAM ELEVATION (FEET) = 2100.00 DOWNSTREAM ELEVATION (FEET) = 2060.00  
 STREET LENGTH (FEET) = 476.59 CURB HEIGHT (INCHES) = 6.0  
 STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00  
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 162.69  
 \*\*\*STREET FLOWING FULL\*\*\*  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH (FEET) = 0.67  
 HALFSTREET FLOOD WIDTH (FEET) = 26.25  
 AVERAGE FLOW VELOCITY (FEET/SEC.) = 11.21  
 PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 7.45  
 STREET FLOW TRAVEL TIME (MIN.) = 0.71 Tc (MIN.) = 31.67  
 \* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.811  
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	4.27	0.75	0.700	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	5.25	0.61	1.000	66
RESIDENTIAL					



"3-4 DWELLINGS/ACRE" B 1.13 0.75 0.600 56  
 SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.67  
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.837  
 SUBAREA AREA(ACRES) = 10.65 SUBAREA RUNOFF(CFS) = 11.99  
 EFFECTIVE AREA(ACRES) = 145.01 AREA-AVERAGED Fm(INCH/HR) = 0.54  
 AREA-AVERAGED Fp(INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.81  
 TOTAL AREA(ACRES) = 145.0 PEAK FLOW RATE(CFS) = 165.69

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.67 HALFSTREET FLOOD WIDTH(FEET) = 26.43  
 FLOW VELOCITY(FEET/SEC.) = 11.26 DEPTH\*VELOCITY(FT\*FT/SEC.) = 7.53  
 LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20312.00 = 6575.98 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20312.00 TO NODE 20313.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 2060.00 DOWNSTREAM ELEVATION(FEET) = 2040.00  
 STREET LENGTH(FEET) = 500.29 CURB HEIGHT(INCHES) = 6.0  
 STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 171.52  
 \*\*\*STREET FLOWING FULL\*\*\*  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.76  
 HALFSTREET FLOOD WIDTH(FEET) = 30.89  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 8.66  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 6.56  
 STREET FLOW TRAVEL TIME(MIN.) = 0.96 Tc(MIN.) = 32.63  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.778  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	6.45	0.75	0.700	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	1.19	0.75	0.600	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	2.82	0.61	1.000	66

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.70  
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.770  
 SUBAREA AREA(ACRES) = 10.46 SUBAREA RUNOFF(CFS) = 11.66  
 EFFECTIVE AREA(ACRES) = 155.47 AREA-AVERAGED Fm(INCH/HR) = 0.54  
 AREA-AVERAGED Fp(INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.81

TOTAL AREA(ACRES) = 155.5 PEAK FLOW RATE(CFS) = 173.14

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.76 HALFSTREET FLOOD WIDTH(FEET) = 31.01  
 FLOW VELOCITY(FEET/SEC.) = 8.67 DEPTH\*VELOCITY(FT\*FT/SEC.) = 6.59  
 LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20313.00 = 7076.27 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20313.00 TO NODE 20314.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 2040.00 DOWNSTREAM ELEVATION(FEET) = 2020.00  
 STREET LENGTH(FEET) = 462.82 CURB HEIGHT(INCHES) = 6.0  
 STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 178.90  
 \*\*\*STREET FLOWING FULL\*\*\*  
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.76  
 HALFSTREET FLOOD WIDTH(FEET) = 30.95  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 8.99  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 6.83  
 STREET FLOW TRAVEL TIME(MIN.) = 0.86 Tc(MIN.) = 33.49  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.751

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER					
"OPEN BRUSH"	B	3.76	0.61	1.000	66
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	5.77	0.75	0.700	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	1.10	0.75	0.600	56

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.69  
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.796  
 SUBAREA AREA(ACRES) = 10.63 SUBAREA RUNOFF(CFS) = 11.51  
 EFFECTIVE AREA(ACRES) = 166.10 AREA-AVERAGED Fm(INCH/HR) = 0.54  
 AREA-AVERAGED Fp(INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.81  
 TOTAL AREA(ACRES) = 166.1 PEAK FLOW RATE(CFS) = 180.81

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.32

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.76 HALFSTREET FLOOD WIDTH (FEET) = 31.07  
FLOW VELOCITY (FEET/SEC.) = 9.02 DEPTH\*VELOCITY (FT\*FT/SEC.) = 6.87  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20314.00 = 7539.09 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20314.00 TO NODE 20315.00 IS CODE = 62

-----  
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<

-----  
UPSTREAM ELEVATION (FEET) = 2020.00 DOWNSTREAM ELEVATION (FEET) = 1980.00  
STREET LENGTH (FEET) = 511.41 CURB HEIGHT (INCHES) = 6.0  
STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00  
INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 185.76  
\*\*\*STREET FLOWING FULL\*\*\*  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH (FEET) = 0.70  
HALFSTREET FLOOD WIDTH (FEET) = 27.96  
AVERAGE FLOW VELOCITY (FEET/SEC.) = 11.35  
PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 7.94  
STREET FLOW TRAVEL TIME (MIN.) = 0.75 Tc (MIN.) = 34.24  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.728  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	6.85	0.75	0.700	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	1.24	0.75	0.600	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	1.05	0.61	1.000	66

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.73  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.721  
SUBAREA AREA (ACRES) = 9.14 SUBAREA RUNOFF (CFS) = 9.90  
EFFECTIVE AREA (ACRES) = 175.24 AREA-AVERAGED Fm (INCH/HR) = 0.54  
AREA-AVERAGED Fp (INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.80  
TOTAL AREA (ACRES) = 175.2 PEAK FLOW RATE (CFS) = 187.25

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.01

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH (FEET) = 0.70 HALFSTREET FLOOD WIDTH (FEET) = 28.08  
FLOW VELOCITY (FEET/SEC.) = 11.34 DEPTH\*VELOCITY (FT\*FT/SEC.) = 7.96  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20315.00 = 8050.50 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20315.00 TO NODE 20316.00 IS CODE = 62

-----  
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<

-----  
UPSTREAM ELEVATION (FEET) = 1980.00 DOWNSTREAM ELEVATION (FEET) = 1950.00  
STREET LENGTH (FEET) = 522.61 CURB HEIGHT (INCHES) = 6.0  
STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00  
INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 191.21  
\*\*\*STREET FLOWING FULL\*\*\*  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH (FEET) = 0.74  
HALFSTREET FLOOD WIDTH (FEET) = 30.04  
AVERAGE FLOW VELOCITY (FEET/SEC.) = 10.18  
PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 7.54  
STREET FLOW TRAVEL TIME (MIN.) = 0.86 Tc (MIN.) = 35.09  
\* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.702  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	6.12	0.75	0.700	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	1.25	0.75	0.600	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.683  
SUBAREA AREA (ACRES) = 7.37 SUBAREA RUNOFF (CFS) = 7.90  
EFFECTIVE AREA (ACRES) = 182.61 AREA-AVERAGED Fm (INCH/HR) = 0.54  
AREA-AVERAGED Fp (INCH/HR) = 0.68 AREA-AVERAGED Ap = 0.80  
TOTAL AREA (ACRES) = 182.6 PEAK FLOW RATE (CFS) = 191.15

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.62

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH (FEET) = 0.74 HALFSTREET FLOOD WIDTH (FEET) = 30.04  
FLOW VELOCITY (FEET/SEC.) = 10.18 DEPTH\*VELOCITY (FT\*FT/SEC.) = 7.54  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20316.00 = 8573.11 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20316.00 TO NODE 20317.00 IS CODE = 62

-----  
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<

-----  
UPSTREAM ELEVATION (FEET) = 1950.00 DOWNSTREAM ELEVATION (FEET) = 1890.00  
STREET LENGTH (FEET) = 743.58 CURB HEIGHT (INCHES) = 6.0  
STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 194.13  
\*\*\*STREET FLOWING FULL\*\*\*  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 0.71  
HALFSTREET FLOOD WIDTH(FEET) = 28.27  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 11.62  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 8.19  
STREET FLOW TRAVEL TIME(MIN.) = 1.07 Tc(MIN.) = 36.16  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.672  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"2 DWELLINGS/ACRE" B 4.10 0.75 0.700 56  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 1.55 0.75 0.600 56  
RESIDENTIAL  
".4 DWELLING/ACRE" B 0.01 0.75 0.900 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.673  
SUBAREA AREA(ACRES) = 5.66 SUBAREA RUNOFF(CFS) = 5.95  
EFFECTIVE AREA(ACRES) = 188.27 AREA-AVERAGED Fm(INCH/HR) = 0.54  
AREA-AVERAGED Fp(INCH/HR) = 0.68 AREA-AVERAGED Ap = 0.79  
TOTAL AREA(ACRES) = 188.3 PEAK FLOW RATE(CFS) = 192.12

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 0.70 HALFSTREET FLOOD WIDTH(FEET) = 28.20  
FLOW VELOCITY(FEET/SEC.) = 11.54 DEPTH\*VELOCITY(FT\*FT/SEC.) = 8.13  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20317.00 = 9316.69 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20317.00 TO NODE 20318.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<  
=====

UPSTREAM ELEVATION(FEET) = 1890.00 DOWNSTREAM ELEVATION(FEET) = 1860.00  
STREET LENGTH(FEET) = 640.63 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 198.21  
\*\*\*STREET FLOWING FULL\*\*\*  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 0.77  
HALFSTREET FLOOD WIDTH(FEET) = 31.74  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 9.49  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 7.35  
STREET FLOW TRAVEL TIME(MIN.) = 1.13 Tc(MIN.) = 37.29  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.642  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 1.10 0.75 0.600 56  
RESIDENTIAL  
".4 DWELLING/ACRE" B 0.01 0.75 0.900 56  
RESIDENTIAL  
"2 DWELLINGS/ACRE" B 10.92 0.75 0.700 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.691  
SUBAREA AREA(ACRES) = 12.03 SUBAREA RUNOFF(CFS) = 12.18  
EFFECTIVE AREA(ACRES) = 200.30 AREA-AVERAGED Fm(INCH/HR) = 0.54  
AREA-AVERAGED Fp(INCH/HR) = 0.68 AREA-AVERAGED Ap = 0.79  
TOTAL AREA(ACRES) = 200.3 PEAK FLOW RATE(CFS) = 199.14

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 0.78 HALFSTREET FLOOD WIDTH(FEET) = 31.81  
FLOW VELOCITY(FEET/SEC.) = 9.50 DEPTH\*VELOCITY(FT\*FT/SEC.) = 7.37  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20318.00 = 9957.32 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20318.00 TO NODE 20319.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 18 USED)<<<<<  
=====

UPSTREAM ELEVATION(FEET) = 1860.00 DOWNSTREAM ELEVATION(FEET) = 1835.00  
STREET LENGTH(FEET) = 624.00 CURB HEIGHT(INCHES) = 8.0  
STREET HALFWIDTH(FEET) = 26.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 15.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 275.14  
\*\*\*STREET FLOWING FULL\*\*\*  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.91  
 HALFSTREET FLOOD WIDTH(FEET) = 38.14  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 9.53  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 8.67  
 STREET FLOW TRAVEL TIME(MIN.) = 1.09 Tc(MIN.) = 38.38  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.613  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	1.46	0.75	0.600	56
RESIDENTIAL ".4 DWELLING/ACRE"	B	9.05	0.75	0.900	56
RESIDENTIAL "2 DWELLINGS/ACRE"	B	100.00	0.75	0.700	56
RESIDENTIAL "2 DWELLINGS/ACRE"	B	28.82	0.75	0.700	56
NATURAL FAIR COVER "OPEN BRUSH"	B	18.27	0.61	1.000	66

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.73  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.745  
 SUBAREA AREA(ACRES) = 157.60 SUBAREA RUNOFF(CFS) = 151.98  
 EFFECTIVE AREA(ACRES) = 357.90 AREA-AVERAGED Fm(INCH/HR) = 0.54  
 AREA-AVERAGED Fp(INCH/HR) = 0.70 AREA-AVERAGED Ap = 0.77  
 TOTAL AREA(ACRES) = 357.9 PEAK FLOW RATE(CFS) = 346.04

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.68

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.98 HALFSTREET FLOOD WIDTH(FEET) = 41.44  
 FLOW VELOCITY(FEET/SEC.) = 10.14 DEPTH\*VELOCITY(FT\*FT/SEC.) = 9.89  
 \*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 624.0 FT WITH ELEVATION-DROP = 25.0 FT, IS 427.3 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20319.00  
 LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20319.00 = 10581.32 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20319.00 TO NODE 20330.00 IS CODE = 62

-----  
 >>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 18 USED)<<<<<  
 =====  
 UPSTREAM ELEVATION(FEET) = 1835.00 DOWNSTREAM ELEVATION(FEET) = 1813.00  
 STREET LENGTH(FEET) = 597.75 CURB HEIGHT(INCHES) = 8.0  
 STREET HALFWIDTH(FEET) = 26.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 15.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 347.61  
 \*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
 STREET FLOW DEPTH(FEET) = 0.99  
 HALFSTREET FLOOD WIDTH(FEET) = 42.17  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 9.83  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 9.74  
 STREET FLOW TRAVEL TIME(MIN.) = 1.01 Tc(MIN.) = 39.39  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.588  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	0.71	0.75	0.600	56
RESIDENTIAL ".4 DWELLING/ACRE"	B	2.91	0.75	0.900	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.841  
 SUBAREA AREA(ACRES) = 3.62 SUBAREA RUNOFF(CFS) = 3.13  
 EFFECTIVE AREA(ACRES) = 361.52 AREA-AVERAGED Fm(INCH/HR) = 0.54  
 AREA-AVERAGED Fp(INCH/HR) = 0.70 AREA-AVERAGED Ap = 0.77  
 TOTAL AREA(ACRES) = 361.5 PEAK FLOW RATE(CFS) = 346.04  
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.99 HALFSTREET FLOOD WIDTH(FEET) = 42.11  
 FLOW VELOCITY(FEET/SEC.) = 9.82 DEPTH\*VELOCITY(FT\*FT/SEC.) = 9.71  
 LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20330.00 = 11179.07 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20330.00 TO NODE 20330.00 IS CODE = 1

-----  
 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 =====  
 TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION(MIN.) = 39.39  
 RAINFALL INTENSITY(INCH/HR) = 1.59  
 AREA-AVERAGED Fm(INCH/HR) = 0.54  
 AREA-AVERAGED Fp(INCH/HR) = 0.70  
 AREA-AVERAGED Ap = 0.77  
 EFFECTIVE STREAM AREA(ACRES) = 361.52  
 TOTAL STREAM AREA(ACRES) = 361.52  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 346.04

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20320.00 TO NODE 20321.00 IS CODE = 21

-----  
 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
 >>>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<  
 =====  
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 1020.45  
 ELEVATION DATA: UPSTREAM(FEET) = 2240.00 DOWNSTREAM(FEET) = 2180.00  
 Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20  
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 19.882  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.394

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
NATURAL FAIR COVER "OPEN BRUSH"	B	9.71	0.61	1.000	66	19.88

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.61  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 1.000  
SUBAREA RUNOFF(CFS) = 15.56  
TOTAL AREA(ACRES) = 9.71 PEAK FLOW RATE(CFS) = 15.56

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 7.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20321.00 TO NODE 20322.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<  
=====

ELEVATION DATA: UPSTREAM(FEET) = 2180.00 DOWNSTREAM(FEET) = 2160.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 548.49 CHANNEL SLOPE = 0.0365  
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 1.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 15.56  
FLOW VELOCITY(FEET/SEC.) = 2.11 FLOW DEPTH(FEET) = 0.38  
TRAVEL TIME(MIN.) = 4.34 Tc(MIN.) = 24.22  
LONGEST FLOWPATH FROM NODE 20320.00 TO NODE 20322.00 = 1568.94 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20322.00 TO NODE 20322.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<  
=====

MAINLINE Tc(MIN.) = 24.22  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.127  
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL FAIR COVER "OPEN BRUSH"	B	15.34	0.61	1.000	66
RESIDENTIAL "2 DWELLINGS/ACRE"	B	0.02	0.75	0.700	56

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.61  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 1.000  
SUBAREA AREA(ACRES) = 15.36 SUBAREA RUNOFF(CFS) = 20.91  
EFFECTIVE AREA(ACRES) = 25.07 AREA-AVERAGED Fm(INCH/HR) = 0.61  
AREA-AVERAGED Fp(INCH/HR) = 0.61 AREA-AVERAGED Ap = 1.00  
TOTAL AREA(ACRES) = 25.1 PEAK FLOW RATE(CFS) = 34.13

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.45; 30M = 0.92; 1HR = 1.21; 3HR = 2.00; 6HR = 2.75; 24HR = 7.29

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20322.00 TO NODE 20323.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<  
=====

ELEVATION DATA: UPSTREAM(FEET) = 2160.00 DOWNSTREAM(FEET) = 2150.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 479.58 CHANNEL SLOPE = 0.0209  
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 34.13  
FLOW VELOCITY(FEET/SEC.) = 2.06 FLOW DEPTH(FEET) = 0.58  
TRAVEL TIME(MIN.) = 3.87 Tc(MIN.) = 28.09  
LONGEST FLOWPATH FROM NODE 20320.00 TO NODE 20323.00 = 2048.52 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20323.00 TO NODE 20323.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<  
=====

MAINLINE Tc(MIN.) = 28.09  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.946  
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "2 DWELLINGS/ACRE"	B	11.74	0.75	0.700	56
NATURAL FAIR COVER "OPEN BRUSH"	B	8.32	0.61	1.000	66

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.68  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.824  
SUBAREA AREA(ACRES) = 20.06 SUBAREA RUNOFF(CFS) = 24.99  
EFFECTIVE AREA(ACRES) = 45.13 AREA-AVERAGED Fm(INCH/HR) = 0.59  
AREA-AVERAGED Fp(INCH/HR) = 0.64 AREA-AVERAGED Ap = 0.92  
TOTAL AREA(ACRES) = 45.1 PEAK FLOW RATE(CFS) = 55.04

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.45; 30M = 0.92; 1HR = 1.21; 3HR = 2.00; 6HR = 2.75; 24HR = 6.70

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20323.00 TO NODE 20324.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<  
=====

ELEVATION DATA: UPSTREAM(FEET) = 2150.00 DOWNSTREAM(FEET) = 2100.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 676.85 CHANNEL SLOPE = 0.0739  
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 55.04  
FLOW VELOCITY(FEET/SEC.) = 3.79 FLOW DEPTH(FEET) = 0.54  
TRAVEL TIME(MIN.) = 2.98 Tc(MIN.) = 31.07  
LONGEST FLOWPATH FROM NODE 20320.00 TO NODE 20324.00 = 2725.37 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20324.00 TO NODE 20324.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<  
=====

MAINLINE Tc(MIN.) = 31.07  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.832  
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
-------------------------------	-------------------	-----------------	-----------------	-----------------	-----------

RESIDENTIAL  
"2 DWELLINGS/ACRE" B 14.74 0.75 0.700 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.700  
SUBAREA AREA(ACRES) = 14.74 SUBAREA RUNOFF(CFS) = 17.35  
EFFECTIVE AREA(ACRES) = 59.87 AREA-AVERAGED Fm(INCH/HR) = 0.57  
AREA-AVERAGED Fp(INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.87  
TOTAL AREA(ACRES) = 59.9 PEAK FLOW RATE(CFS) = 67.76

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.93; 1HR = 1.23; 3HR = 2.02; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20324.00 TO NODE 20325.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

-----  
ELEVATION DATA: UPSTREAM(FEET) = 2100.00 DOWNSTREAM(FEET) = 2080.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 631.62 CHANNEL SLOPE = 0.0317  
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 67.76  
FLOW VELOCITY(FEET/SEC.) = 2.90 FLOW DEPTH(FEET) = 0.68  
TRAVEL TIME(MIN.) = 3.63 Tc(MIN.) = 34.70  
LONGEST FLOWPATH FROM NODE 20320.00 TO NODE 20325.00 = 3356.99 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20325.00 TO NODE 20325.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

-----  
MAINLINE Tc(MIN.) = 34.70  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.714  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"2 DWELLINGS/ACRE" B 10.91 0.75 0.700 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.700  
SUBAREA AREA(ACRES) = 10.91 SUBAREA RUNOFF(CFS) = 11.69  
EFFECTIVE AREA(ACRES) = 70.78 AREA-AVERAGED Fm(INCH/HR) = 0.57  
AREA-AVERAGED Fp(INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.84  
TOTAL AREA(ACRES) = 70.8 PEAK FLOW RATE(CFS) = 73.11

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20325.00 TO NODE 20326.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

-----  
ELEVATION DATA: UPSTREAM(FEET) = 2080.00 DOWNSTREAM(FEET) = 2050.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 686.64 CHANNEL SLOPE = 0.0437  
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000

MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 73.11  
FLOW VELOCITY(FEET/SEC.) = 3.29 FLOW DEPTH(FEET) = 0.67  
TRAVEL TIME(MIN.) = 3.48 Tc(MIN.) = 38.18  
LONGEST FLOWPATH FROM NODE 20320.00 TO NODE 20326.00 = 4043.63 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20326.00 TO NODE 20326.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

-----  
MAINLINE Tc(MIN.) = 38.18  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.618  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"2 DWELLINGS/ACRE" B 48.19 0.75 0.700 56  
RESIDENTIAL  
"3-4 DWELLINGS/ACRE" B 0.06 0.75 0.600 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.700  
SUBAREA AREA(ACRES) = 48.25 SUBAREA RUNOFF(CFS) = 47.55  
EFFECTIVE AREA(ACRES) = 119.03 AREA-AVERAGED Fm(INCH/HR) = 0.55  
AREA-AVERAGED Fp(INCH/HR) = 0.70 AREA-AVERAGED Ap = 0.78  
TOTAL AREA(ACRES) = 119.0 PEAK FLOW RATE(CFS) = 114.58

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.44; 30M = 0.91; 1HR = 1.20; 3HR = 2.00; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20326.00 TO NODE 20327.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

-----  
ELEVATION DATA: UPSTREAM(FEET) = 2050.00 DOWNSTREAM(FEET) = 1990.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 1389.79 CHANNEL SLOPE = 0.0432  
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 3.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 114.58  
FLOW VELOCITY(FEET/SEC.) = 3.68 FLOW DEPTH(FEET) = 0.79  
TRAVEL TIME(MIN.) = 6.29 Tc(MIN.) = 44.47  
LONGEST FLOWPATH FROM NODE 20320.00 TO NODE 20327.00 = 5433.42 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20327.00 TO NODE 20327.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

-----  
MAINLINE Tc(MIN.) = 44.47  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.477  
SUBAREA LOSS RATE DATA(AMC II):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
RESIDENTIAL  
"2 DWELLINGS/ACRE" B 16.19 0.75 0.700 56  
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.700  
SUBAREA AREA(ACRES) = 16.19 SUBAREA RUNOFF(CFS) = 13.89  
EFFECTIVE AREA(ACRES) = 135.22 AREA-AVERAGED Fm(INCH/HR) = 0.55  
AREA-AVERAGED Fp(INCH/HR) = 0.71 AREA-AVERAGED Ap = 0.77  
TOTAL AREA(ACRES) = 135.2 PEAK FLOW RATE(CFS) = 114.58  
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20327.00 TO NODE 20328.00 IS CODE = 54  
-----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	1990.00	DOWNSTREAM(FEET) =	1920.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	1079.99	CHANNEL SLOPE =	0.0648
CHANNEL BASE(FEET) =	0.00	"Z" FACTOR =	50.000
MANNING'S FACTOR =	0.045	MAXIMUM DEPTH(FEET) =	3.00
CHANNEL FLOW THRU SUBAREA(CFS) =	114.58		
FLOW VELOCITY(FEET/SEC.) =	4.31	FLOW DEPTH(FEET) =	0.73
TRAVEL TIME(MIN.) =	4.18	Tc(MIN.) =	48.65
LONGEST FLOWPATH FROM NODE 20320.00 TO NODE 20328.00 =	6513.41 FEET.		

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20328.00 TO NODE 20328.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) =	48.65				
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	1.399				
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS
LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	25.33	0.75	0.700	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.27	0.75	0.600	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =	0.75				
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =	0.699				
SUBAREA AREA(ACRES) =	25.60	SUBAREA RUNOFF(CFS) =	20.20		
EFFECTIVE AREA(ACRES) =	160.82	AREA-AVERAGED Fm(INCH/HR) =	0.54		
AREA-AVERAGED Fp(INCH/HR) =	0.71	AREA-AVERAGED Ap =	0.76		
TOTAL AREA(ACRES) =	160.8	PEAK FLOW RATE(CFS) =	124.08		

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20328.00 TO NODE 20329.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) =	1920.00	DOWNSTREAM ELEVATION(FEET) =	1870.00
STREET LENGTH(FEET) =	1075.25	CURB HEIGHT(INCHES) =	6.0
STREET HALFWIDTH(FEET) =	18.00		

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 129.32

\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) =	0.68		
HALFSTREET FLOOD WIDTH(FEET) =	26.92		
AVERAGE FLOW VELOCITY(FEET/SEC.) =	8.49		
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) =	5.76		
STREET FLOW TRAVEL TIME(MIN.) =	2.11	Tc(MIN.) =	50.76
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =	1.364		

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	13.84	0.75	0.700	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.700

SUBAREA AREA(ACRES) =	13.84	SUBAREA RUNOFF(CFS) =	10.47
EFFECTIVE AREA(ACRES) =	174.66	AREA-AVERAGED Fm(INCH/HR) =	0.54
AREA-AVERAGED Fp(INCH/HR) =	0.71	AREA-AVERAGED Ap =	0.76
TOTAL AREA(ACRES) =	174.7	PEAK FLOW RATE(CFS) =	129.46

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) =	0.68	HALFSTREET FLOOD WIDTH(FEET) =	26.92
FLOW VELOCITY(FEET/SEC.) =	8.50	DEPTH*VELOCITY(FT*FT/SEC.) =	5.77
LONGEST FLOWPATH FROM NODE 20320.00 TO NODE 20329.00 =	7588.66 FEET.		

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20329.00 TO NODE 20330.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) =	1870.00	DOWNSTREAM ELEVATION(FEET) =	1813.00
STREET LENGTH(FEET) =	927.52	CURB HEIGHT(INCHES) =	6.0
STREET HALFWIDTH(FEET) =	18.00		

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curb-to-curb) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

```

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 135.55
***STREET FLOWING FULL***
STRETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.66
HALFSTREET FLOOD WIDTH(FEET) = 25.95
AVERAGE FLOW VELOCITY(FEET/SEC.) = 9.55
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 6.29
STREET FLOW TRAVEL TIME(MIN.) = 1.62 Tc(MIN.) = 52.38
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.339
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B 0.48 0.75 0.600 56
RESIDENTIAL
".4 DWELLING/ACRE" B 5.88 0.75 0.900 56
RESIDENTIAL
"2 DWELLINGS/ACRE" B 11.27 0.75 0.700 56
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.764
SUBAREA AREA(ACRES) = 17.63 SUBAREA RUNOFF(CFS) = 12.18
EFFECTIVE AREA(ACRES) = 192.29 AREA-AVERAGED Fm(INCH/HR) = 0.54
AREA-AVERAGED Fp(INCH/HR) = 0.72 AREA-AVERAGED Ap = 0.76
TOTAL AREA(ACRES) = 192.3 PEAK FLOW RATE(CFS) = 137.63

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.66 HALFSTREET FLOOD WIDTH(FEET) = 26.13
FLOW VELOCITY(FEET/SEC.) = 9.57 DEPTH*VELOCITY(FT*FT/SEC.) = 6.34
LONGEST FLOWPATH FROM NODE 20320.00 TO NODE 20330.00 = 8516.18 FEET.

*****
FLOW PROCESS FROM NODE 20330.00 TO NODE 20330.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 52.38
RAINFALL INTENSITY(INCH/HR) = 1.34
AREA-AVERAGED Fm(INCH/HR) = 0.54
AREA-AVERAGED Fp(INCH/HR) = 0.72
AREA-AVERAGED Ap = 0.76
EFFECTIVE STREAM AREA(ACRES) = 192.29
TOTAL STREAM AREA(ACRES) = 192.29
PEAK FLOW RATE(CFS) AT CONFLUENCE = 137.63

** CONFLUENCE DATA **
STREAM Q Tc Intensity Fp(Fm) Ap Ae HEADWATER
NUMBER (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES) NODE
1 346.04 39.39 1.588 0.70( 0.54) 0.77 361.5 20300.00
2 137.63 52.38 1.339 0.72( 0.54) 0.76 192.3 20320.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

```

```

** PEAK FLOW RATE TABLE **
STREAM Q Tc Intensity Fp(Fm) Ap Ae HEADWATER
NUMBER (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES) NODE
1 482.04 39.39 1.588 0.71( 0.54) 0.77 506.1 20300.00
2 401.28 52.38 1.339 0.71( 0.54) 0.77 553.8 20320.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 482.04 Tc(MIN.) = 39.39
EFFECTIVE AREA(ACRES) = 506.13 AREA-AVERAGED Fm(INCH/HR) = 0.54
AREA-AVERAGED Fp(INCH/HR) = 0.71 AREA-AVERAGED Ap = 0.77
TOTAL AREA(ACRES) = 553.8
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20330.00 = 11179.07 FEET.

*****
FLOW PROCESS FROM NODE 20330.00 TO NODE 20349.00 IS CODE = 62
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STREET TABLE SECTION # 18 USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 1813.00 DOWNSTREAM ELEVATION(FEET) = 1785.00
STREET LENGTH(FEET) = 1334.61 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 26.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 15.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 488.27
***STREET FLOWING FULL***
STRETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 1.21
HALFSTREET FLOOD WIDTH(FEET) = 53.22
AVERAGE FLOW VELOCITY(FEET/SEC.) = 8.65
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 10.48
STREET FLOW TRAVEL TIME(MIN.) = 2.57 Tc(MIN.) = 41.96
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.529
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B 1.05 0.75 0.600 56
RESIDENTIAL
"2 DWELLINGS/ACRE" B 12.65 0.75 0.700 56
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.692
SUBAREA AREA(ACRES) = 13.70 SUBAREA RUNOFF(CFS) = 12.47
EFFECTIVE AREA(ACRES) = 519.83 AREA-AVERAGED Fm(INCH/HR) = 0.54
AREA-AVERAGED Fp(INCH/HR) = 0.71 AREA-AVERAGED Ap = 0.76
TOTAL AREA(ACRES) = 567.5 PEAK FLOW RATE(CFS) = 482.04
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):

```



5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 1.21 HALFSTREET FLOOD WIDTH(FEET) = 52.97  
FLOW VELOCITY(FEET/SEC.) = 8.62 DEPTH\*VELOCITY(FT\*FT/SEC.) = 10.40  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20349.00 = 12513.68 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20349.00 TO NODE 20349.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 41.96  
RAINFALL INTENSITY(INCH/HR) = 1.53  
AREA-AVERAGED Fm(INCH/HR) = 0.54  
AREA-AVERAGED Fp(INCH/HR) = 0.71  
AREA-AVERAGED Ap = 0.76  
EFFECTIVE STREAM AREA(ACRES) = 519.83  
TOTAL STREAM AREA(ACRES) = 567.51  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 482.04

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20340.00 TO NODE 20341.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 998.88  
ELEVATION DATA: UPSTREAM(FEET) = 2120.00 DOWNSTREAM(FEET) = 2080.00

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20  
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.422  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.175  
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
NATURAL FAIR COVER						
"GRASS"	B	6.76	0.57	1.000	69	21.29
RESIDENTIAL						
"3-4 DWELLINGS/ACRE"	B	1.12	0.75	0.600	56	12.42

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.58  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.943  
SUBAREA RUNOFF(CFS) = 18.62  
TOTAL AREA(ACRES) = 7.88 PEAK FLOW RATE(CFS) = 18.62

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.43; 30M = 0.88; 1HR = 1.16; 3HR = 1.97; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20341.00 TO NODE 20342.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 2080.00 DOWNSTREAM(FEET) = 2055.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 397.26 CHANNEL SLOPE = 0.0629

CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 18.62  
FLOW VELOCITY(FEET/SEC.) = 2.71 FLOW DEPTH(FEET) = 0.37  
TRAVEL TIME(MIN.) = 2.45 Tc(MIN.) = 14.87  
LONGEST FLOWPATH FROM NODE 20340.00 TO NODE 20342.00 = 1396.14 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20342.00 TO NODE 20342.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 14.87  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.850  
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	4.25	0.75	0.700	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.25	0.75	0.600	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.694  
SUBAREA AREA(ACRES) = 4.50 SUBAREA RUNOFF(CFS) = 9.44  
EFFECTIVE AREA(ACRES) = 12.38 AREA-AVERAGED Fm(INCH/HR) = 0.54  
AREA-AVERAGED Fp(INCH/HR) = 0.63 AREA-AVERAGED Ap = 0.85  
TOTAL AREA(ACRES) = 12.4 PEAK FLOW RATE(CFS) = 25.75

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.43; 30M = 0.88; 1HR = 1.16; 3HR = 1.97; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20342.00 TO NODE 20343.00 IS CODE = 54

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 2055.00 DOWNSTREAM(FEET) = 2035.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 438.38 CHANNEL SLOPE = 0.0456  
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
CHANNEL FLOW THRU SUBAREA(CFS) = 25.75  
FLOW VELOCITY(FEET/SEC.) = 2.60 FLOW DEPTH(FEET) = 0.45  
TRAVEL TIME(MIN.) = 2.81 Tc(MIN.) = 17.68  
LONGEST FLOWPATH FROM NODE 20340.00 TO NODE 20343.00 = 1834.52 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20343.00 TO NODE 20343.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 17.68  
\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.568  
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	5.37	0.75	0.700	56

RESIDENTIAL  
 "3-4 DWELLINGS/ACRE" B 0.37 0.75 0.600 56  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.694  
 SUBAREA AREA(ACRES) = 5.74 SUBAREA RUNOFF(CFS) = 10.59  
 EFFECTIVE AREA(ACRES) = 18.12 AREA-AVERAGED Fm(INCH/HR) = 0.53  
 AREA-AVERAGED Fp(INCH/HR) = 0.66 AREA-AVERAGED Ap = 0.80  
 TOTAL AREA(ACRES) = 18.1 PEAK FLOW RATE(CFS) = 33.21

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.44; 30M = 0.89; 1HR = 1.18; 3HR = 1.98; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20343.00 TO NODE 20344.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 2035.00 DOWNSTREAM(FEET) = 2015.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 496.72 CHANNEL SLOPE = 0.0403  
 CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
 CHANNEL FLOW THRU SUBAREA(CFS) = 33.21  
 FLOW VELOCITY(FEET/SEC.) = 2.65 FLOW DEPTH(FEET) = 0.50  
 TRAVEL TIME(MIN.) = 3.12 Tc(MIN.) = 20.81  
 LONGEST FLOWPATH FROM NODE 20340.00 TO NODE 20344.00 = 2331.24 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20344.00 TO NODE 20344.00 IS CODE = 81  
 -----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 20.81  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.330  
 SUBAREA LOSS RATE DATA(AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	2.06	0.75	0.700	56
RESIDENTIAL					
".4 DWELLING/ACRE"	B	2.77	0.75	0.900	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.07	0.75	0.600	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.812  
 SUBAREA AREA(ACRES) = 4.90 SUBAREA RUNOFF(CFS) = 7.60  
 EFFECTIVE AREA(ACRES) = 23.02 AREA-AVERAGED Fm(INCH/HR) = 0.55  
 AREA-AVERAGED Fp(INCH/HR) = 0.68 AREA-AVERAGED Ap = 0.80  
 TOTAL AREA(ACRES) = 23.0 PEAK FLOW RATE(CFS) = 36.91

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.45; 30M = 0.92; 1HR = 1.21; 3HR = 2.00; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20344.00 TO NODE 20345.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 2015.00 DOWNSTREAM(FEET) = 1980.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 575.06 CHANNEL SLOPE = 0.0609  
 CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
 CHANNEL FLOW THRU SUBAREA(CFS) = 36.91  
 FLOW VELOCITY(FEET/SEC.) = 3.16 FLOW DEPTH(FEET) = 0.48  
 TRAVEL TIME(MIN.) = 3.04 Tc(MIN.) = 23.84  
 LONGEST FLOWPATH FROM NODE 20340.00 TO NODE 20345.00 = 2906.30 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20345.00 TO NODE 20345.00 IS CODE = 81  
 -----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 23.84  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.147  
 SUBAREA LOSS RATE DATA(AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	12.00	0.75	0.700	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.27	0.75	0.600	56
RESIDENTIAL					
".4 DWELLING/ACRE"	B	3.29	0.75	0.900	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.741  
 SUBAREA AREA(ACRES) = 15.56 SUBAREA RUNOFF(CFS) = 22.31  
 EFFECTIVE AREA(ACRES) = 38.58 AREA-AVERAGED Fm(INCH/HR) = 0.55  
 AREA-AVERAGED Fp(INCH/HR) = 0.71 AREA-AVERAGED Ap = 0.78  
 TOTAL AREA(ACRES) = 38.6 PEAK FLOW RATE(CFS) = 55.43

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20345.00 TO NODE 20346.00 IS CODE = 54  
 -----

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1980.00 DOWNSTREAM(FEET) = 1940.00  
 CHANNEL LENGTH THRU SUBAREA(FEET) = 558.59 CHANNEL SLOPE = 0.0716  
 CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000  
 MANNING'S FACTOR = 0.045 MAXIMUM DEPTH(FEET) = 2.00  
 CHANNEL FLOW THRU SUBAREA(CFS) = 55.43  
 FLOW VELOCITY(FEET/SEC.) = 3.71 FLOW DEPTH(FEET) = 0.55  
 TRAVEL TIME(MIN.) = 2.51 Tc(MIN.) = 26.35  
 LONGEST FLOWPATH FROM NODE 20340.00 TO NODE 20346.00 = 3464.89 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20346.00 TO NODE 20346.00 IS CODE = 81  
 -----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 26.35  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.022

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	3.53	0.75	0.700	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.62	0.75	0.600	56
RESIDENTIAL					
".4 DWELLING/ACRE"	B	3.41	0.75	0.900	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.782  
SUBAREA AREA(ACRES) = 7.56 SUBAREA RUNOFF(CFS) = 9.78  
EFFECTIVE AREA(ACRES) = 46.14 AREA-AVERAGED Fm(INCH/HR) = 0.56  
AREA-AVERAGED Fp(INCH/HR) = 0.71 AREA-AVERAGED Ap = 0.78  
TOTAL AREA(ACRES) = 46.1 PEAK FLOW RATE(CFS) = 60.86

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20346.00 TO NODE 20347.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 1940.00 DOWNSTREAM ELEVATION(FEET) = 1890.00  
STREET LENGTH(FEET) = 993.62 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 67.37

\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.56  
HALFSTREET FLOOD WIDTH(FEET) = 20.76  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 7.20  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 4.00

STREET FLOW TRAVEL TIME(MIN.) = 2.30 Tc(MIN.) = 28.65

\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.923

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	2.71	0.75	0.600	56
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	6.04	0.75	0.700	56
RESIDENTIAL					
".4 DWELLING/ACRE"	B	1.62	0.75	0.900	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.705

SUBAREA AREA(ACRES) = 10.37	SUBAREA RUNOFF(CFS) = 13.02
EFFECTIVE AREA(ACRES) = 56.51	AREA-AVERAGED Fm(INCH/HR) = 0.55
AREA-AVERAGED Fp(INCH/HR) = 0.72	AREA-AVERAGED Ap = 0.77
TOTAL AREA(ACRES) = 56.5	PEAK FLOW RATE(CFS) = 69.78

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.56 HALFSTREET FLOOD WIDTH(FEET) = 21.00  
FLOW VELOCITY(FEET/SEC.) = 7.30 DEPTH\*VELOCITY(FT\*FT/SEC.) = 4.09  
LONGEST FLOWPATH FROM NODE 20340.00 TO NODE 20347.00 = 4458.51 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20347.00 TO NODE 20348.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 1890.00 DOWNSTREAM ELEVATION(FEET) = 1860.00  
STREET LENGTH(FEET) = 874.50 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 77.76

\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.61  
HALFSTREET FLOOD WIDTH(FEET) = 23.51  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 6.60  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 4.03  
STREET FLOW TRAVEL TIME(MIN.) = 2.21 Tc(MIN.) = 30.86

\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.839

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.78	0.75	0.600	56
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	12.66	0.75	0.700	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.694  
SUBAREA AREA(ACRES) = 13.44 SUBAREA RUNOFF(CFS) = 15.96  
EFFECTIVE AREA(ACRES) = 69.95 AREA-AVERAGED Fm(INCH/HR) = 0.54  
AREA-AVERAGED Fp(INCH/HR) = 0.72 AREA-AVERAGED Ap = 0.75  
TOTAL AREA(ACRES) = 69.9 PEAK FLOW RATE(CFS) = 81.48

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.62 HALFSTREET FLOOD WIDTH(FEET) = 23.87  
 FLOW VELOCITY(FEET/SEC.) = 6.72 DEPTH\*VELOCITY(FT\*FT/SEC.) = 4.15  
 LONGEST FLOWPATH FROM NODE 20340.00 TO NODE 20348.00 = 5333.01 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20348.00 TO NODE 20349.00 IS CODE = 62  
 -----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<  
 =====

UPSTREAM ELEVATION(FEET) = 1860.00 DOWNSTREAM ELEVATION(FEET) = 1785.00  
 STREET LENGTH(FEET) = 1082.38 CURB HEIGHT(INCHES) = 6.0  
 STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALfstREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 100.39  
 \*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.59  
 HALFSTREET FLOOD WIDTH(FEET) = 22.65  
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 9.13  
 PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 5.42  
 STREET FLOW TRAVEL TIME(MIN.) = 1.98 Tc(MIN.) = 32.83  
 \* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.772

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"2 DWELLINGS/ACRE"	B	33.09	0.75	0.700	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	0.55	0.75	0.600	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.698  
 SUBAREA AREA(ACRES) = 33.64 SUBAREA RUNOFF(CFS) = 37.83  
 EFFECTIVE AREA(ACRES) = 103.59 AREA-AVERAGED Fm(INCH/HR) = 0.54  
 AREA-AVERAGED Fp(INCH/HR) = 0.73 AREA-AVERAGED Ap = 0.73  
 TOTAL AREA(ACRES) = 103.6 PEAK FLOW RATE(CFS) = 115.07

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.62 HALFSTREET FLOOD WIDTH(FEET) = 23.81  
 FLOW VELOCITY(FEET/SEC.) = 9.53 DEPTH\*VELOCITY(FT\*FT/SEC.) = 5.87  
 \*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 1082.4 FT WITH ELEVATION-DROP = 75.0 FT, IS 84.9 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20349.00  
 LONGEST FLOWPATH FROM NODE 20340.00 TO NODE 20349.00 = 6415.39 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20349.00 TO NODE 20349.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<  
 =====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION(MIN.) = 32.83  
 RAINFALL INTENSITY(INCH/HR) = 1.77  
 AREA-AVERAGED Fm(INCH/HR) = 0.54  
 AREA-AVERAGED Fp(INCH/HR) = 0.73  
 AREA-AVERAGED Ap = 0.73  
 EFFECTIVE STREAM AREA(ACRES) = 103.59  
 TOTAL STREAM AREA(ACRES) = 103.59  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 115.07

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	482.04	41.96	1.529	0.71( 0.54)	0.76	519.8	20300.00
1	401.28	55.06	1.299	0.71( 0.54)	0.76	567.5	20320.00
2	115.07	32.83	1.772	0.73( 0.54)	0.73	103.6	20340.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	584.75	32.83	1.772	0.71( 0.54)	0.76	510.4	20340.00
2	574.51	41.96	1.529	0.71( 0.54)	0.76	623.4	20300.00
3	472.29	55.06	1.299	0.71( 0.54)	0.76	671.1	20320.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 584.75 Tc(MIN.) = 32.83  
 EFFECTIVE AREA(ACRES) = 510.35 AREA-AVERAGED Fm(INCH/HR) = 0.54  
 AREA-AVERAGED Fp(INCH/HR) = 0.71 AREA-AVERAGED Ap = 0.76  
 TOTAL AREA(ACRES) = 671.1  
 LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20349.00 = 12513.68 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20349.00 TO NODE 20349.00 IS CODE = 71  
 -----

>>>>PEAK FLOW RATE ESTIMATOR CHANGED TO UNIT-HYDROGRAPH METHOD<<<<<  
 >>>>USING TIME-OF-CONCENTRATION OF LONGEST FLOWPATH<<<<<  
 =====

UNIT-HYDROGRAPH DATA:

RAINFALL(INCH): 5M= 0.46;30M= 0.94;1H= 1.24;3H= 2.02;6H= 2.75;24H= 6.83  
 S-GRAPH: VALLEY(DEV.)= 75.1%;VALLEY(UNDEV.)/DESERT= 24.9%  
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%  
 Tc(HR) = 0.70; LAG(HR) = 0.56; Fm(INCH/HR) = 0.54; Ybar = 0.50  
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.  
 DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;  
 3HR = 1.00; 6HR = 1.00; 24HR= 1.00  
 UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 671.1  
 LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20349.00 = 12513.68 FEET.  
 EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3,n=.0580; Lca/L=0.4,n=.0520; Lca/L=0.5,n=.0478;Lca/L=0.6,n=.0446  
 TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 200.99  
 UNIT-HYDROGRAPH METHOD PEAK FLOW RATE (CFS) = 686.96  
 TOTAL PEAK FLOW RATE (CFS) = 686.96 (SOURCE FLOW INCLUDED)  
 RATIONAL METHOD PEAK FLOW RATE (CFS) = 584.75  
 (UPSTREAM NODE PEAK FLOW RATE (CFS) = 584.75)  
 PEAK FLOW RATE (CFS) USED = 686.96

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20349.00 TO NODE 20350.00 IS CODE = 62  
 -----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<

=====

UPSTREAM ELEVATION (FEET) = 1785.00 DOWNSTREAM ELEVATION (FEET) = 1715.00  
 STREET LENGTH (FEET) = 1290.16 CURB HEIGHT (INCHES) = 6.0  
 STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00  
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 715.76  
 \*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 1.16  
 HALFSTREET FLOOD WIDTH (FEET) = 51.22  
 AVERAGE FLOW VELOCITY (FEET/SEC.) = 13.46  
 PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 15.67  
 STREET FLOW TRAVEL TIME (MIN.) = 1.60 Tc (MIN.) = 43.56  
 \* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.495

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	4.52	0.75	0.600	56
RESIDENTIAL ".4 DWELLING/ACRE"	B	72.05	0.75	0.900	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.75

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.882

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M= 0.46;30M= 0.94;1H= 1.24;3H= 2.02;6H= 2.75;24H= 6.80

S-GRAPH: VALLEY (DEV.)= 68.0%;VALLEY (UNDEV.)/DESERT= 32.0%

MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT (UNDEV.)= 0.0%

Tc (HR) = 0.73; LAG (HR) = 0.58; Fm (INCH/HR) = 0.55; Ybar = 0.52

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;

3HR = 0.99; 6HR = 1.00; 24HR= 1.00

UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 747.7

LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20350.00 = 13803.84 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3,n=.0557; Lca/L=0.4,n=.0499; Lca/L=0.5,n=.0459;Lca/L=0.6,n=.0428

TIME OF PEAK FLOW (HR) = 16.58 RUNOFF VOLUME (AF) = 217.79

UNIT-HYDROGRAPH PEAK FLOW RATE (CFS) = 718.83  
 TOTAL AREA (ACRES) = 747.7 PEAK FLOW RATE (CFS) = 718.83

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):  
 5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 1.17 HALFSTREET FLOOD WIDTH (FEET) = 51.28  
 FLOW VELOCITY (FEET/SEC.) = 13.48 DEPTH\*VELOCITY (FT\*FT/SEC.) = 15.71

\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
 AND L = 1290.2 FT WITH ELEVATION-DROP = 70.0 FT, IS 167.9 CFS,  
 WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20350.00

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 20350.00 TO NODE 20351.00 IS CODE = 62  
 -----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>(STREET TABLE SECTION # 5 USED)<<<<<

=====

UPSTREAM ELEVATION (FEET) = 1715.00 DOWNSTREAM ELEVATION (FEET) = 1680.00  
 STREET LENGTH (FEET) = 1342.03 CURB HEIGHT (INCHES) = 6.0  
 STREET HALFWIDTH (FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK (FEET) = 10.00  
 INSIDE STREET CROSSFALL (DECIMAL) = 0.020  
 OUTSIDE STREET CROSSFALL (DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
 STREET PARKWAY CROSSFALL (DECIMAL) = 0.020  
 Manning's FRICTION FACTOR for Streetflow Section (curb-to-curb) = 0.0180  
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 747.49  
 \*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH (FEET) = 1.34  
 HALFSTREET FLOOD WIDTH (FEET) = 60.25  
 AVERAGE FLOW VELOCITY (FEET/SEC.) = 10.19  
 PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 13.71  
 STREET FLOW TRAVEL TIME (MIN.) = 2.19 Tc (MIN.) = 45.75  
 \* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 1.452

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	7.14	0.75	0.600	56
RESIDENTIAL ".4 DWELLING/ACRE"	B	72.56	0.75	0.900	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.75

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.873

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M= 0.46;30M= 0.94;1H= 1.24;3H= 2.02;6H= 2.75;24H= 6.77

S-GRAPH: VALLEY (DEV.)= 62.4%;VALLEY (UNDEV.)/DESERT= 37.6%

MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT (UNDEV.)= 0.0%

Tc (HR) = 0.76; LAG (HR) = 0.61; Fm (INCH/HR) = 0.56; Ybar = 0.53

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;

3HR = 0.99; 6HR = 1.00; 24HR= 1.00

UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 827.4  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20351.00 = 15145.87 FEET.  
EQUIVALENT BASIN FACTOR APPROXIMATIONS:  
Lca/L=0.3,n=.0540; Lca/L=0.4,n=.0484; Lca/L=0.5,n=.0445;Lca/L=0.6,n=.0415  
TIME OF PEAK FLOW(HR) = 16.67 RUNOFF VOLUME(AF) = 235.54  
UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 757.04  
TOTAL AREA(ACRES) = 827.4 PEAK FLOW RATE(CFS) = 757.04

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 1.35 HALFSTREET FLOOD WIDTH(FEET) = 60.55  
FLOW VELOCITY(FEET/SEC.) = 10.22 DEPTH\*VELOCITY(FT\*FT/SEC.) = 13.81

\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
AND L = 1342.0 FT WITH ELEVATION-DROP = 35.0 FT, IS 154.6 CFS,  
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20351.00

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20351.00 TO NODE 20352.00 IS CODE = 62  
-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STREET TABLE SECTION # 5 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 1680.00 DOWNSTREAM ELEVATION(FEET) = 1655.00  
STREET LENGTH(FEET) = 1091.03 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0180  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 762.63

\*\*\*STREET FLOWING FULL\*\*\*

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 1.39  
HALFSTREET FLOOD WIDTH(FEET) = 62.32  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 9.73  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 13.48

STREET FLOW TRAVEL TIME(MIN.) = 1.87 Tc(MIN.) = 47.62

\* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.417

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL ".4 DWELLING/ACRE"	B	15.77	0.75	0.900	56
RESIDENTIAL "3-4 DWELLINGS/ACRE"	B	0.71	0.75	0.600	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.887

UNIT-HYDROGRAPH DATA:

RAINFALL(INCH): 5M= 0.46;30M= 0.94;1H= 1.24;3H= 2.02;6H= 2.75;24H= 6.77

S-GRAPH: VALLEY(DEV.)= 61.2%;VALLEY(UNDEV.)/DESERT= 38.8%

MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%  
Tc(HR) = 0.79; LAG(HR) = 0.63; Fm(INCH/HR) = 0.56; Ybar = 0.53  
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.  
DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;  
3HR = 0.99; 6HR = 1.00; 24HR= 1.00  
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 843.8  
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20352.00 = 16236.90 FEET.  
EQUIVALENT BASIN FACTOR APPROXIMATIONS:  
Lca/L=0.3,n=.0529; Lca/L=0.4,n=.0474; Lca/L=0.5,n=.0435;Lca/L=0.6,n=.0406  
TIME OF PEAK FLOW(HR) = 16.67 RUNOFF VOLUME(AF) = 239.13  
UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 756.39  
TOTAL AREA(ACRES) = 843.8 PEAK FLOW RATE(CFS) = 757.04  
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):  
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 1.38 HALFSTREET FLOOD WIDTH(FEET) = 62.14  
FLOW VELOCITY(FEET/SEC.) = 9.71 DEPTH\*VELOCITY(FT\*FT/SEC.) = 13.43

\*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,  
AND L = 1091.0 FT WITH ELEVATION-DROP = 25.0 FT, IS 33.3 CFS,  
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 20352.00

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20352.00 TO NODE 20352.00 IS CODE = 10  
-----

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

=====

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20274.00 TO NODE 20274.00 IS CODE = 15.1  
-----

>>>>DEFINE MEMORY BANK # 2 <<<<<

=====

PEAK FLOWRATE TABLE FILE NAME: 20274.DNA

MEMORY BANK # 2 DEFINED AS FOLLOWS:

PEAK FLOW RATE(CFS) = 2478.03 Tc(MIN.) = 41.02

AREA-AVERAGED Fm(INCH/HR) = 0.59 Ybar = 0.54

TOTAL AREA(ACRES) = 3101.9

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20274.00 = 19473.89 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20274.00 TO NODE 20274.00 IS CODE = 14.0  
-----

>>>>MEMORY BANK # 2 COPIED ONTO MAIN-STREAM MEMORY<<<<<

=====

MAIN-STREAM MEMORY DEFINED AS FOLLOWS:

PEAK FLOW RATE(CFS) = 2478.03 Tc(MIN.) = 41.02

AREA-AVERAGED Fm(INCH/HR) = 0.59 Ybar = 0.54

TOTAL AREA(ACRES) = 3101.9

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20274.00 = 19473.89 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 20274.00 TO NODE 20274.00 IS CODE = 12  
-----

>>>>CLEAR MEMORY BANK # 2 <<<<<

```

*****
FLOW PROCESS FROM NODE 20274.00 TO NODE 20352.00 IS CODE = 54
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 1670.00 DOWNSTREAM(FEET) = 1655.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 623.43 CHANNEL SLOPE = 0.0241
CHANNEL BASE(FEET) = 10.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.035 MAXIMUM DEPTH(FEET) = 10.00
CHANNEL FLOW THRU SUBAREA(CFS) = 2478.03
FLOW VELOCITY(FEET/SEC.) = 16.23 FLOW DEPTH(FEET) = 6.59
TRAVEL TIME(MIN.) = 0.64 Tc(MIN.) = 41.66
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20352.00 = 20097.32 FEET.
*****
FLOW PROCESS FROM NODE 20352.00 TO NODE 20352.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
MAINLINE Tc(MIN.) = 41.66
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.536
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
SCHOOL B 10.49 0.75 0.600 56
RESIDENTIAL
"3-4 DWELLINGS/ACRE" B 1.59 0.75 0.600 56
RESIDENTIAL
".4 DWELLING/ACRE" B 21.45 0.75 0.900 56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.792
SUBAREA AREA(ACRES) = 33.53
UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.45;30M= 0.92;1H= 1.21;3H= 2.05;6H= 2.87;24H= 7.06
S-GRAPH: VALLEY(DEV.)= 35.0%;VALLEY(UNDEV.)/DESERT= 65.0%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.69; LAG(HR) = 0.56; Fm(INCH/HR) = 0.59; Ybar = 0.54
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.86; 30M = 0.86; 1HR = 0.86;
3HR = 0.98; 6HR = 0.99; 24HR= 0.99
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 3135.5
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20352.00 = 20097.32 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0412; Lca/L=0.4,n=.0369; Lca/L=0.5,n=.0339;Lca/L=0.6,n=.0316
TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 884.32
UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 2480.59
TOTAL AREA(ACRES) = 3135.5 PEAK FLOW RATE(CFS) = 2480.59

SUBAREA AREA-AVERAGED RAINFALL DEPTH(INCH):
5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50
*****
FLOW PROCESS FROM NODE 20352.00 TO NODE 20352.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<

```

```

*****
** MAIN STREAM CONFLUENCE DATA **
PEAK FLOW RATE(CFS) = 2480.59 Tc(MIN.) = 41.66
AREA-AVERAGED Fm(INCH/HR) = 0.59 Ybar = 0.54
TOTAL AREA(ACRES) = 3135.5
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20352.00 = 20097.32 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **
PEAK FLOW RATE(CFS) = 757.04 Tc(MIN.) = 47.62
AREA-AVERAGED Fm(INCH/HR) = 0.56 Ybar = 0.53
TOTAL AREA(ACRES) = 843.8
LONGEST FLOWPATH FROM NODE 20300.00 TO NODE 20352.00 = 16236.90 FEET.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.45;30M= 0.92;1H= 1.22;3H= 2.05;6H= 2.84;24H= 6.99
S-GRAPH: VALLEY(DEV.)= 40.6%;VALLEY(UNDEV.)/DESERT= 59.4%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.69; LAG(HR) = 0.56; Fm(INCH/HR) = 0.58; Ybar = 0.54
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.82; 30M = 0.82; 1HR = 0.82;
3HR = 0.97; 6HR = 0.99; 24HR= 0.99
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 3979.3
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20352.00 = 20097.32 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0412; Lca/L=0.4,n=.0369; Lca/L=0.5,n=.0339;Lca/L=0.6,n=.0316
TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 1112.54
PEAK FLOW RATE(CFS) = 3054.49
*****
FLOW PROCESS FROM NODE 20352.00 TO NODE 20352.00 IS CODE = 12
-----
>>>>CLEAR MEMORY BANK # 1 <<<<
-----
*****
FLOW PROCESS FROM NODE 20352.00 TO NODE 20353.00 IS CODE = 54
-----
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
-----
ELEVATION DATA: UPSTREAM(FEET) = 1655.00 DOWNSTREAM(FEET) = 1625.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 1454.79 CHANNEL SLOPE = 0.0206
CHANNEL BASE(FEET) = 12.00 "Z" FACTOR = 2.000
MANNING'S FACTOR = 0.035 MAXIMUM DEPTH(FEET) = 10.00
CHANNEL FLOW THRU SUBAREA(CFS) = 3054.49
FLOW VELOCITY(FEET/SEC.) = 16.10 FLOW DEPTH(FEET) = 7.19
TRAVEL TIME(MIN.) = 1.51 Tc(MIN.) = 43.16
LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20353.00 = 21552.11 FEET.

*****
FLOW PROCESS FROM NODE 20353.00 TO NODE 20353.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
-----
MAINLINE Tc(MIN.) = 43.16
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.504

```

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
SCHOOL	B	20.64	0.75	0.600	56
RESIDENTIAL					
"3-4 DWELLINGS/ACRE"	B	1.09	0.75	0.600	56
RESIDENTIAL					
".4 DWELLING/ACRE"	B	25.75	0.75	0.900	56
NATURAL FAIR COVER					
"OPEN BRUSH"	B	2.69	0.61	1.000	66

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.74

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.775

SUBAREA AREA (ACRES) = 50.17

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M= 0.45; 30M= 0.93; 1H= 1.22; 3H= 2.05; 6H= 2.84; 24H= 6.99

S-GRAPH: VALLEY (DEV.)= 40.6%; VALLEY (UNDEV.)/DESERT= 59.4%

MOUNTAIN= 0.0%; FOOTHILL= 0.0%; DESERT (UNDEV.)= 0.0%

Tc (HR) = 0.72; LAG (HR) = 0.58; Fm (INCH/HR) = 0.58; Ybar = 0.54

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.82; 30M = 0.82; 1HR = 0.82;

3HR = 0.97; 6HR = 0.99; 24HR= 0.99

UNIT-INTERVAL (MIN) = 5.00 TOTAL AREA (ACRES) = 4029.5

LONGEST FLOWPATH FROM NODE 20120.00 TO NODE 20353.00 = 21552.11 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3, n=.0401; Lca/L=0.4, n=.0359; Lca/L=0.5, n=.0330; Lca/L=0.6, n=.0308

TIME OF PEAK FLOW (HR) = 16.58 RUNOFF VOLUME (AF) = 1124.97

UNIT-HYDROGRAPH PEAK FLOW RATE (CFS) = 3007.68

TOTAL AREA (ACRES) = 4029.5 PEAK FLOW RATE (CFS) = 3054.49

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

SUBAREA AREA-AVERAGED RAINFALL DEPTH (INCH):

5M = 0.46; 30M = 0.95; 1HR = 1.25; 3HR = 2.03; 6HR = 2.75; 24HR = 6.50



\*\*\*\*\*

FLOOD ROUTING ANALYSIS  
USING COUNTY HYDROLOGY MANUAL OF SAN BERNARDINO(1986)  
(c) Copyright 1989-2011 Advanced Engineering Software (aes)  
Ver. 18.0 Release Date: 05/01/2011 License ID 1224

Analysis prepared by:

San Bernardino County  
Public Works Department

Water Resources Division

\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* Opal Basin Routing W/O Crafton Basin \*  
\* Opal Basin Feasibility Study \*  
\* Node 20353 AMC-III \*  
\*\*\*\*\*

FILE NAME: OPB353A.DAT  
TIME/DATE OF STUDY: 13:19 09/12/2013

\*\*\*\*\*  
FLOW PROCESS FROM NODE 250.00 TO NODE 250.00 IS CODE = 1  
\*\*\*\*\*

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<

(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERSHED AREA = 2221.150 ACRES  
BASEFLOW = 0.000 CFS/SQUARE-MILE  
\*USER ENTERED "LAG" TIME = 0.490 HOURS  
CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.  
THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)  
MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.  
VALLEY(DEVELOPED):  
"S"-CURVE PERCENTAGE(DECIMAL NOTATION) = 0.384  
FOOTHILL "S"-CURVE PERCENTAGE(DECIMAL NOTATION) = 0.000  
MOUNTAIN "S"-CURVE PERCENTAGE(DECIMAL NOTATION) = 0.000  
VALLEY(UNDEVELOPED)/DESERT:  
"S"-CURVE PERCENTAGE(DECIMAL NOTATION) = 0.616  
DESERT(UNDEVELOPED) "S"-CURVE PERCENTAGE(DECIMAL NOTATION) = 0.000  
MAXIMUM WATERSHED LOSS RATE (INCH/HOUR) = 0.350  
LOW LOSS FRACTION = 0.310  
\*HYDROGRAPH MODEL #1 SPECIFIED\*

SPECIFIED PEAK 5-MINUTES RAINFALL(INCH)= 0.46  
SPECIFIED PEAK 30-MINUTES RAINFALL(INCH)= 0.94  
SPECIFIED PEAK 1-HOUR RAINFALL(INCH) = 1.24  
SPECIFIED PEAK 3-HOUR RAINFALL(INCH) = 2.13  
SPECIFIED PEAK 6-HOUR RAINFALL(INCH) = 3.01  
SPECIFIED PEAK 24-HOUR RAINFALL(INCH) = 7.33

\*USER SPECIFIED PRECIPITATION DEPTH-AREA REDUCTION FACTORS:

5-MINUTE FACTOR = 0.820  
30-MINUTE FACTOR = 0.820  
1-HOUR FACTOR = 0.820  
3-HOUR FACTOR = 0.973  
6-HOUR FACTOR = 0.987  
24-HOUR FACTOR = 0.992

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES  
UNIT INTERVAL PERCENTAGE OF LAG-TIME = 17.007

RUNOFF HYDROGRAPH LISTING LIMITS:  
MODEL TIME(HOURS) FOR BEGINNING OF RESULTS = 16.00  
MODEL TIME(HOURS) FOR END OF RESULTS = 19.00

UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES (CFS)
1	1.264	339.471
2	4.631	904.392
3	10.785	1653.291
4	20.280	2550.490
5	31.798	3093.866
6	44.681	3460.766
7	56.611	3204.613
8	65.772	2460.882
9	72.515	1811.287
10	77.424	1318.574
11	81.028	968.023
12	83.809	747.209
13	86.000	588.511
14	87.702	457.146
15	88.983	344.116
16	90.061	289.541
17	91.003	253.192
18	91.867	232.069
19	92.618	201.551
20	93.356	198.299
21	94.000	172.960
22	94.489	131.369
23	94.972	129.678
24	95.384	110.695
25	95.772	104.231
26	96.159	104.063
27	96.498	90.910
28	96.806	82.778
29	97.114	82.722
30	97.421	82.589
31	97.664	65.255
32	97.867	54.654
33	98.071	54.615
34	98.274	54.654
35	98.478	54.615

36	98.681	54.617
37	98.811	34.926
38	98.886	20.219
39	98.961	20.183
40	99.037	20.217
41	99.112	20.183
42	99.187	20.219
43	99.262	20.183
44	99.337	20.181
45	99.413	20.219
46	99.488	20.181
47	99.563	20.183
48	99.638	20.181
49	99.713	20.181
50	99.788	20.183
51	99.863	20.181
52	99.938	20.183
53	100.000	16.537

-----  
TOTAL SOIL-LOSS VOLUME (ACRE-FEET) = 399.4406  
TOTAL STORM RUNOFF VOLUME (ACRE-FEET) = 945.8401  
-----

=====

2 4 - H O U R   S T O R M  
R U N O F F   H Y D R O G R A P H

=====

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)  
(Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	575.0	1150.0	1725.0	2300.0
16.000	531.0939	870.28	.	.	Q	. V	.
16.083	538.3464	1053.07	.	.	Q	. V	.
16.167	547.4697	1324.70	.	.	.	Q	.
16.250	558.8416	1651.20	.	.	.	V	Q
16.333	572.5420	1989.30	.	.	.	V	Q
16.417	587.5893	2184.86	.	.	.	V	Q
16.500	603.2255	2270.37	.	.	.	V	Q
16.583	617.9086	2132.00	.	.	.	V	Q
16.667	630.5999	1842.77	.	.	.	V	Q
16.750	641.5264	1586.54	.	.	.	Q	.
16.833	651.1153	1392.30	.	.	.	Q	V
16.917	659.7304	1250.91	.	.	.	Q	V
17.000	667.6461	1149.35	.	.	.	Q	V
17.083	674.9822	1065.21	.	.	.	Q	V
17.167	681.7884	988.26	.	.	.	Q	V
17.250	688.0996	916.39	.	.	.	Q	V
17.333	694.0256	860.45	.	.	.	Q	V
17.417	699.6032	809.88	.	.	.	Q	V
17.500	704.8595	763.21	.	.	.	Q	V
17.583	709.7971	716.93	.	.	.	Q	V
17.667	714.4987	682.67	.	.	.	Q	V

17.750	718.9498	646.31	.	.Q	.	V	.
17.833	723.1519	610.14	.	Q	.	V	.
17.917	727.2042	588.38	.	Q	.	V	.
18.000	731.0951	564.97	.	Q	.	V	.
18.083	734.8704	548.16	.	Q	.	.V	.
18.167	738.5620	536.02	.	Q	.	.V	.
18.250	742.1741	524.48	.	Q	.	.V	.
18.333	745.7465	518.72	.	Q	.	.V	.
18.417	749.3156	518.22	.	Q	.	.V	.
18.500	752.8924	519.36	.	Q	.	.V	.
18.583	756.4420	515.40	.	Q	.	.V	.
18.667	759.9651	511.57	.	Q	.	. V	.
18.750	763.4725	509.27	.	Q	.	. V	.
18.833	766.9568	505.91	.	Q	.	. V	.
18.917	770.4078	501.09	.	Q	.	. V	.
19.000	773.8166	494.95	.	Q	.	. V	.

-----  
TIME DURATION (minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
(Note: 100% of Peak Flow Rate estimate assumed to have  
an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1145.0
10%	1145.0
20%	400.0
30%	175.0
40%	75.0
50%	55.0
60%	40.0
70%	30.0
80%	25.0
90%	15.0

\*\*\*\*\*  
FLOW PROCESS FROM NODE 250.00 TO NODE 274.00 IS CODE = 5.2  
-----

>>>>MODEL CHANNEL ROUTING OF STREAM #1 BY THE CONVEX METHOD<<<<  
-----

THE MODIFIED C-ROUTING COEFFICIENT IS ESTIMATED IN ORDER  
TO ROUTE THE STREAM 1 INFLOW HYDROGRAPH BY 5-MINUTE  
INTERVALS (Reference: the National Engineering Handbook,  
Hydrology, Chapter 17, page 17-52, August, 1972,  
U.S. Department of Commerce).

ASSUMED REGULAR CHANNEL INFORMATION:  
BASEWIDTH (FT) = 30.00      CHANNEL Z = 2.50  
UPSTREAM ELEVATION (FT) = 1733.00  
DOWNSTREAM ELEVATION (FT) = 1670.00  
CHANNEL LENGTH (FT) = 2379.03      MANNING'S FACTOR = 0.035  
CONSTANT LOSS RATE (CFS) = 0.00

CHANNEL ROUTING COEFFICIENT ESTIMATED:

MAXIMUM INFLOW(CFS) = 2270.37  
 AVERAGE FLOWRATE IN EXCESS OF 50% MAXIMUM INFLOW = 1706.76  
 CHANNEL NORMAL VELOCITY FOR Q = 1706.76 CFS = 13.28 FPS  
 ESTIMATED CHANNEL ROUTING COEFFICIENT = 0.887

MODIFIED CHANNEL ROUTING COEFFICIENT FOR 5-MINUTE  
 UNIT INTERVALS IS CSTAR = 0.969

CONVEX METHOD CHANNEL ROUTING RESULTS:

MODEL TIME (HRS)	INFLOW (STREAM 1) (CFS)	ROUTED FLOW (CFS)	OUTFLOW LESS
			LOSS (STREAM 1) (CFS)
16.000	870.28	842.01	842.01
16.083	1053.07	952.73	952.73
16.167	1324.70	1173.77	1173.77
16.250	1651.20	1468.83	1468.83
16.333	1989.30	1799.64	1799.64
16.417	2184.86	2072.53	2072.53
16.500	2270.37	2220.34	2220.34
16.583	2132.00	2205.72	2205.72
16.667	1842.77	2002.44	2002.44
16.750	1586.54	1730.94	1730.94
16.833	1392.30	1502.50	1502.50
16.917	1250.91	1331.29	1331.29
17.000	1149.35	1207.12	1207.12
17.083	1065.21	1112.80	1112.80
17.167	988.26	1031.61	1031.61
17.250	916.39	956.85	956.85
17.333	860.45	892.15	892.15
17.417	809.88	838.38	838.38
17.500	763.21	789.49	789.49
17.583	716.93	742.93	742.93
17.667	682.67	702.13	702.13
17.750	646.31	666.70	666.70
17.833	610.14	630.46	630.46
17.917	588.38	600.86	600.86
18.000	564.97	578.10	578.10
18.083	548.16	557.72	557.72
18.167	536.02	542.93	542.93
18.250	524.48	530.98	530.98
18.333	518.72	522.06	522.06
18.417	518.22	518.60	518.60
18.500	519.36	518.75	518.75
18.583	515.40	517.53	517.53
18.667	511.57	513.72	513.72
18.750	509.27	510.59	510.59
18.833	505.91	507.78	507.78
18.917	501.09	503.78	503.78
19.000	494.95	498.38	498.38

PROCESS SUMMARY OF STORAGE:

INFLOW VOLUME = 945.840 AF  
 OUTFLOW VOLUME = 945.840 AF  
 LOSS VOLUME = 0.000 AF

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 274.00 TO NODE 274.00 IS CODE = 1  
 -----  
 >>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<  
 =====

(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERSHED AREA = 880.780 ACRES  
 BASEFLOW = 0.000 CFS/SQUARE-MILE  
 \*USER ENTERED "LAG" TIME = 0.440 HOURS  
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.  
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)  
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.  
 VALLEY (DEVELOPED):  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.266  
 FOOTHILL "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 MOUNTAIN "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 VALLEY (UNDEVELOPED) / DESERT:  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.730  
 DESERT (UNDEVELOPED) "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.004  
 MAXIMUM WATERSHED LOSS RATE (INCH/HOUR) = 0.357  
 LOW LOSS FRACTION = 0.338  
 \*HYDROGRAPH MODEL #1 SPECIFIED\*

SPECIFIED PEAK 5-MINUTES RAINFALL (INCH) = 0.41  
 SPECIFIED PEAK 30-MINUTES RAINFALL (INCH) = 0.85  
 SPECIFIED PEAK 1-HOUR RAINFALL (INCH) = 1.12  
 SPECIFIED PEAK 3-HOUR RAINFALL (INCH) = 1.82  
 SPECIFIED PEAK 6-HOUR RAINFALL (INCH) = 2.47  
 SPECIFIED PEAK 24-HOUR RAINFALL (INCH) = 6.29

\*USER SPECIFIED PRECIPITATION DEPTH-AREA REDUCTION FACTORS:  
 5-MINUTE FACTOR = 0.820  
 30-MINUTE FACTOR = 0.820  
 1-HOUR FACTOR = 0.820  
 3-HOUR FACTOR = 0.973  
 6-HOUR FACTOR = 0.987  
 24-HOUR FACTOR = 0.992

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES  
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 18.939

RUNOFF HYDROGRAPH LISTING LIMITS:

MODEL TIME (HOURS) FOR BEGINNING OF RESULTS = 16.00  
 MODEL TIME (HOURS) FOR END OF RESULTS = 19.00

=====

UNIT HYDROGRAPH DETERMINATION

-----

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES (CFS)
1	1.485	158.232
2	5.716	450.656

3	13.433	821.952
4	24.937	1225.442
5	38.586	1453.903
6	52.403	1471.774
7	62.830	1110.684
8	70.231	788.306
9	75.496	560.876
10	79.347	410.158
11	82.289	313.405
12	84.610	247.203
13	86.461	197.174
14	87.889	152.072
15	89.139	133.241
16	90.249	118.206
17	91.205	101.799
18	92.115	96.919
19	92.916	85.333
20	93.564	69.004
21	94.185	66.192
22	94.710	55.879
23	95.224	54.820
24	95.708	51.506
25	96.119	43.842
26	96.528	43.528
27	96.934	43.276
28	97.251	33.750
29	97.521	28.725
30	97.790	28.695
31	98.060	28.692
32	98.329	28.676
33	98.552	23.781
34	98.658	11.259
35	98.758	10.666
36	98.858	10.681
37	98.958	10.651
38	99.058	10.681
39	99.158	10.649
40	99.258	10.633
41	99.358	10.586
42	99.457	10.569
43	99.556	10.569
44	99.655	10.569
45	99.755	10.569
46	99.854	10.569
47	99.953	10.568
48	100.000	5.013

-----  
TOTAL SOIL-LOSS VOLUME (ACRE-FEET) = 148.0885  
TOTAL STORM RUNOFF VOLUME (ACRE-FEET) = 309.6898  
-----

=====

2 4 - H O U R S T O R M  
R U N O F F H Y D R O G R A P H

=====

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)  
(Note: Time indicated is at END of Each Unit Intervals)

-----

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	225.0	450.0	675.0	900.0
16.000	173.3142	298.76	.	.	Q	.	V
16.083	175.9199	378.35	.	.	Q	.	V
16.167	179.3562	498.95	.	.	.	QV	.
16.250	183.7354	635.86	.	.	.	V	Q
16.333	188.9945	763.63	.	.	.	V	Q
16.417	194.6603	822.67	.	.	.	V	Q
16.500	200.1864	802.39	.	.	.	V	Q
16.583	204.8531	677.62	.	.	.	V	Q
16.667	208.7421	564.68	.	.	.	QV	.
16.750	212.0680	482.93	.	.	.	Q	V
16.833	215.0086	426.97	.	.	.	Q	V
16.917	217.6713	386.62	.	.	.	Q	V
17.000	220.1173	355.16	.	.	.	Q	V
17.083	222.3774	328.17	.	.	.	Q	V
17.167	224.4641	302.99	.	.	.	Q	V
17.250	226.4204	284.05	.	.	.	Q	V
17.333	228.2479	265.35	.	.	.	Q	V
17.417	229.9444	246.34	.	.	.	Q	V
17.500	231.5312	230.41	.	.	.	Q	V
17.583	233.0117	214.96	.	.	.	Q	V
17.667	234.3937	200.67	.	.	.	Q	V
17.750	235.7108	191.25	.	.	.	Q	V
17.833	236.9604	181.43	.	.	.	Q	V
17.917	238.1634	174.68	.	.	.	Q	V
18.000	239.3191	167.80	.	.	.	Q	V
18.083	240.4304	161.36	.	.	.	Q	V
18.167	241.5231	158.66	.	.	.	Q	V
18.250	242.6107	157.92	.	.	.	Q	V
18.333	243.6933	157.20	.	.	.	Q	V
18.417	244.7895	159.16	.	.	.	Q	V
18.500	245.9107	162.80	.	.	.	Q	V
18.583	247.0465	164.92	.	.	.	Q	V
18.667	248.1851	165.32	.	.	.	Q	V
18.750	249.3097	163.29	.	.	.	Q	V
18.833	250.4058	159.15	.	.	.	Q	V
18.917	251.4931	157.88	.	.	.	Q	V
19.000	252.5728	156.76	.	.	.	Q	V

-----

TIME DURATION (minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
(Note: 100% of Peak Flow Rate estimate assumed to have an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1145.0
10%	1145.0
20%	230.0
30%	120.0
40%	60.0
50%	45.0

60% 35.0  
 70% 25.0  
 80% 20.0  
 90% 15.0

\*\*\*\*\*

FLOW PROCESS FROM NODE 274.00 TO NODE 274.00 IS CODE = 11

>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<<

STREAM HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)  
 (Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	775.0	1550.0	2325.0	3100.0
16.000	701.0480	1140.78	.	.	Q	.	V
16.083	710.2152	1331.08	.	.	Q	.	V
16.167	721.7353	1672.71	.	.	.	Q	V
16.250	736.2304	2104.69	.	.	.	V	Q
16.333	753.8838	2563.27	.	.	.	V	Q
16.417	773.8232	2895.20	.	.	.	V	Q
16.500	794.6408	3022.72	.	.	.	V	Q
16.583	814.4985	2883.34	.	.	.	V	Q
16.667	832.1784	2567.12	.	.	.	V	Q
16.750	847.4254	2213.87	.	.	.	V	Q
16.833	860.7138	1929.47	.	.	.	Q	V
16.917	872.5451	1717.91	.	.	.	Q	V
17.000	883.3046	1562.28	.	.	.	Q	V
17.083	893.2286	1440.97	.	.	.	Q	V
17.167	902.4201	1334.60	.	.	.	Q	V
17.250	910.9663	1240.91	.	.	.	Q	V
17.333	918.9380	1157.50	.	.	.	Q	V
17.417	926.4086	1084.72	.	.	.	Q	V
17.500	933.4327	1019.90	.	.	.	Q	V
17.583	940.0297	957.89	.	.	.	Q	V
17.667	946.2473	902.79	.	.	.	Q	V
17.750	952.1561	857.96	.	.	.	Q	V
17.833	957.7477	811.89	.	.	.	Q	V
17.917	963.0889	775.54	.	.	.	Q	V
18.000	968.2260	745.90	.	.	.	Q	V
18.083	973.1782	719.07	.	.	.	Q	V
18.167	978.0101	701.59	.	.	.	Q	V
18.250	982.7545	688.89	.	.	.	Q	V
18.333	987.4326	679.26	.	.	.	Q	V
18.417	992.1003	677.76	.	.	.	Q	V
18.500	996.7943	681.56	.	.	.	Q	V
18.583	1001.4943	682.45	.	.	.	Q	V
18.667	1006.1709	679.04	.	.	.	Q	V
18.750	1010.8120	673.88	.	.	.	Q	V
18.833	1015.4052	666.93	.	.	.	Q	V
18.917	1019.9620	661.66	.	.	.	Q	V
19.000	1024.4740	655.14	.	.	.	Q	V

TIME DURATION (minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
 (Note: 100% of Peak Flow Rate estimate assumed to have an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1145.0
10%	1145.0
20%	390.0
30%	165.0
40%	75.0
50%	55.0
60%	40.0
70%	30.0
80%	25.0
90%	15.0

\*\*\*\*\*

FLOW PROCESS FROM NODE 274.00 TO NODE 352.00 IS CODE = 5.2

>>>>MODEL CHANNEL ROUTING OF STREAM #1 BY THE CONVEX METHOD<<<<<

THE MODIFIED C-ROUTING COEFFICIENT IS ESTIMATED IN ORDER TO ROUTE THE STREAM 1 INFLOW HYDROGRAPH BY 5-MINUTE INTERVALS (Reference: the National Engineering Handbook, Hydrology, Chapter 17, page 17-52, August, 1972, U.S. Department of Commerce).

ASSUMED REGULAR CHANNEL INFORMATION:

BASEWIDTH (FT) = 10.00 CHANNEL Z = 2.00  
 UPSTREAM ELEVATION (FT) = 1670.00  
 DOWNSTREAM ELEVATION (FT) = 1655.00  
 CHANNEL LENGTH (FT) = 623.43 MANNING'S FACTOR = 0.015  
 CONSTANT LOSS RATE (CFS) = 0.00

CHANNEL ROUTING COEFFICIENT ESTIMATED:

MAXIMUM INFLOW (CFS) = 3022.72  
 AVERAGE FLOWRATE IN EXCESS OF 50% MAXIMUM INFLOW = 2284.78  
 CHANNEL NORMAL VELOCITY FOR Q = 2284.78 CFS = 29.68 FPS  
 ESTIMATED CHANNEL ROUTING COEFFICIENT = 0.946

MODIFIED CHANNEL ROUTING COEFFICIENT FOR 5-MINUTE UNIT INTERVALS IS CSTAR = 1.000

CONVEX METHOD CHANNEL ROUTING RESULTS:

MODEL TIME (HRS)	INFLOW (STREAM 1) (CFS)	ROUTED FLOW (CFS)	OUTFLOW LESS LOSS (STREAM 1) (CFS)	
16.000	1140.78	1137.00	1137.00	
16.083	1331.08	1318.48	1318.48	
16.167	1672.71	1650.09	1650.09	
16.250	2104.69	2076.09	2076.09	
16.333	2563.27	2532.91	2532.91	
16.417	2895.20	2873.22	2873.22	

16.500	3022.72	3014.28	3014.28
16.583	2883.34	2892.57	2892.57
16.667	2567.12	2588.06	2588.06
16.750	2213.87	2237.26	2237.26
16.833	1929.47	1948.30	1948.30
16.917	1717.91	1731.92	1731.92
17.000	1562.28	1572.58	1572.58
17.083	1440.97	1449.00	1449.00
17.167	1334.60	1341.65	1341.65
17.250	1240.91	1247.11	1247.11
17.333	1157.50	1163.02	1163.02
17.417	1084.72	1089.54	1089.54
17.500	1019.90	1024.20	1024.20
17.583	957.89	962.00	962.00
17.667	902.79	906.44	906.44
17.750	857.96	860.92	860.92
17.833	811.89	814.94	814.94
17.917	775.54	777.95	777.95
18.000	745.90	747.86	747.86
18.083	719.07	720.85	720.85
18.167	701.59	702.75	702.75
18.250	688.89	689.73	689.73
18.333	679.26	679.89	679.89
18.417	677.76	677.86	677.86
18.500	681.56	681.30	681.30
18.583	682.45	682.39	682.39
18.667	679.04	679.27	679.27
18.750	673.88	674.22	674.22
18.833	666.93	667.39	667.39
18.917	661.66	662.01	662.01
19.000	655.14	655.57	655.57

=====

PROCESS SUMMARY OF STORAGE:

INFLOW VOLUME = 1255.530 AF  
 OUTFLOW VOLUME = 1255.529 AF  
 LOSS VOLUME = 0.000 AF

\*\*\*\*\*

FLOW PROCESS FROM NODE 352.00 TO NODE 352.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<

=====

(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERSHED AREA = 877.380 ACRES  
 BASEFLOW = 0.000 CFS/SQUARE-MILE  
 \*USER ENTERED "LAG" TIME = 0.480 HOURS  
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.  
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)  
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.  
 VALLEY (DEVELOPED):  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.610  
 FOOTHILL "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 MOUNTAIN "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 VALLEY (UNDEVELOPED) / DESERT:  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.390

DESERT (UNDEVELOPED) "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 MAXIMUM WATERSHED LOSS RATE (INCH/HOUR) = 0.330  
 LOW LOSS FRACTION = 0.311  
 \*HYDROGRAPH MODEL #1 SPECIFIED\*

SPECIFIED PEAK 5-MINUTES RAINFALL (INCH) = 0.46  
 SPECIFIED PEAK 30-MINUTES RAINFALL (INCH) = 0.94  
 SPECIFIED PEAK 1-HOUR RAINFALL (INCH) = 1.24  
 SPECIFIED PEAK 3-HOUR RAINFALL (INCH) = 2.02  
 SPECIFIED PEAK 6-HOUR RAINFALL (INCH) = 2.75  
 SPECIFIED PEAK 24-HOUR RAINFALL (INCH) = 6.77

\*USER SPECIFIED PRECIPITATION DEPTH-AREA REDUCTION FACTORS:  
 5-MINUTE FACTOR = 0.820  
 30-MINUTE FACTOR = 0.820  
 1-HOUR FACTOR = 0.820  
 3-HOUR FACTOR = 0.973  
 6-HOUR FACTOR = 0.987  
 24-HOUR FACTOR = 0.992

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES  
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 17.361

RUNOFF HYDROGRAPH LISTING LIMITS:  
 MODEL TIME (HOURS) FOR BEGINNING OF RESULTS = 16.00  
 MODEL TIME (HOURS) FOR END OF RESULTS = 19.00

=====

UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES (CFS)
1	1.182	125.446
2	4.354	336.580
3	10.707	674.107
4	20.574	1046.928
5	32.294	1243.622
6	45.868	1440.305
7	58.683	1359.796
8	69.021	1096.918
9	76.382	781.054
10	81.721	566.459
11	85.386	388.965
12	88.182	296.636
13	90.180	212.057
14	91.687	159.908
15	92.620	99.009
16	93.406	83.359
17	94.116	75.360
18	94.771	69.478
19	95.365	63.026
20	95.959	63.067
21	96.334	39.750
22	96.649	33.412

23	96.945	31.420
24	97.198	26.793
25	97.448	26.610
26	97.688	25.428
27	97.890	21.453
28	98.089	21.127
29	98.288	21.135
30	98.469	19.163
31	98.603	14.199
32	98.734	13.948
33	98.866	13.948
34	98.997	13.957
35	99.129	13.939
36	99.233	11.099
37	99.283	5.316
38	99.332	5.168
39	99.381	5.158
40	99.429	5.149
41	99.478	5.149
42	99.526	5.167
43	99.575	5.140
44	99.624	5.177
45	99.672	5.140
46	99.721	5.140
47	99.769	5.140
48	99.817	5.139
49	99.866	5.140
50	99.914	5.140
51	99.963	5.140
52	100.000	3.952

-----  
TOTAL SOIL-LOSS VOLUME (ACRE-FEET) = 145.4045  
TOTAL STORM RUNOFF VOLUME (ACRE-FEET) = 345.3988  
-----

=====

2 4 - H O U R S T O R M  
R U N O F F H Y D R O G R A P H

=====

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)

(Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	250.0	500.0	750.0	1000.0
16.000	192.7135	335.17	.	.	Q	.	V
16.083	195.5213	407.69	.	.	Q	.	V
16.167	199.0795	516.65	.	.	Q	V	.
16.250	203.6414	662.40	.	.	.	V	Q
16.333	209.1870	805.22	.	.	.	V	Q
16.417	215.2608	881.91	.	.	.	V	Q
16.500	221.6831	932.53	.	.	.	V	Q
16.583	227.7637	882.91	.	.	.	V	Q
16.667	233.0724	770.83	.	.	.	V	Q

16.750	237.5088	644.16	.	.	.	Q	V	.
16.833	241.3148	552.63	.	.	.	Q	V	.
16.917	244.6089	478.30	.	.	.	Q	V	.
17.000	247.5778	431.09	.	.	.	Q	V	.
17.083	250.2464	387.48	.	.	.	Q	V	.
17.167	252.6820	353.64	.	.	.	Q	V	.
17.250	254.8753	318.48	.	.	.	Q	V	.
17.333	256.9194	296.80	.	.	.	Q	V	.
17.417	258.8325	277.78	.	.	.	Q	V	.
17.500	260.6190	259.40	.	.	.	Q	V	.
17.583	262.2833	241.65	.	.	.	Q	V	.
17.667	263.8473	227.09	.	.	.	Q	V	.
17.750	265.2838	208.59	.	.	.	Q	V	.
17.833	266.6406	197.00	.	.	.	Q	V	.
17.917	267.9378	188.36	.	.	.	Q	V	.
18.000	269.1791	180.23	.	.	.	Q	V	.
18.083	270.3817	174.62	.	.	.	Q	V	.
18.167	271.5547	170.32	.	.	.	Q	V	.
18.250	272.7091	167.62	.	.	.	Q	V	.
18.333	273.8674	168.19	.	.	.	Q	V	.
18.417	275.0389	170.10	.	.	.	Q	V	.
18.500	276.2276	172.60	.	.	.	Q	V	.
18.583	277.4275	174.22	.	.	.	Q	V	.
18.667	278.6409	176.18	.	.	.	Q	V	.
18.750	279.8600	177.01	.	.	.	Q	V	.
18.833	281.0784	176.92	.	.	.	Q	V	.
18.917	282.2896	175.86	.	.	.	Q	V	.
19.000	283.4850	173.58	.	.	.	Q	V	.

-----  
TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
(Note: 100% of Peak Flow Rate estimate assumed to have  
an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1145.0
10%	1145.0
20%	215.0
30%	125.0
40%	65.0
50%	50.0
60%	35.0
70%	30.0
80%	25.0
90%	15.0

\*\*\*\*\*

FLOW PROCESS FROM NODE 352.00 TO NODE 352.00 IS CODE = 11

>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<<

STREAM HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)

(Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	1000.0	2000.0	3000.0	4000.0
------------	-------------	---------	----	--------	--------	--------	--------

Time (min)	Inflow (CFS)	Outflow (CFS)	Q	V
16.000	893.2413	1472.17	.	. Q . V .
16.083	905.1295	1726.17	.	. Q . V .
16.167	920.0520	2166.74	.	. . QV .
16.250	938.9121	2738.49	.	. . V Q .
16.333	961.9019	3338.12	.	. . V . Q
16.417	987.7637	3755.13	.	. . V . Q
16.500	1014.9455	3946.80	.	. . V . Q.
16.583	1040.9474	3775.47	.	. . V . Q .
16.667	1064.0802	3358.88	.	. . V . Q .
16.750	1083.9247	2881.42	.	. . VQ .
16.833	1101.1487	2500.93	.	. . Q V .
16.917	1116.3706	2210.22	.	. . Q V .
17.000	1130.1700	2003.67	.	. . Q V .
17.083	1142.8180	1836.48	.	. . Q . V .
17.167	1154.4935	1695.29	.	. . Q . V .
17.250	1165.2758	1565.59	.	. . Q . V .
17.333	1175.3296	1459.82	.	. . Q . V .
17.417	1184.7463	1367.32	.	. . Q . V .
17.500	1193.5865	1283.59	.	. . Q . V .
17.583	1201.8761	1203.65	.	. . Q . V .
17.667	1209.6827	1133.53	.	. . Q . V .
17.750	1217.0486	1069.52	.	. . Q . V .
17.833	1224.0178	1011.94	.	. . Q . V .
17.917	1230.6729	966.31	.	. . Q . V .
18.000	1237.0647	928.09	.	. . Q . V .
18.083	1243.2318	895.47	.	. . Q . V .
18.167	1249.2446	873.06	.	. . Q . V .
18.250	1255.1493	857.35	.	. . Q . V .
18.333	1260.9901	848.09	.	. . Q . V .
18.417	1266.8301	847.96	.	. . Q . V .
18.500	1272.7109	853.90	.	. . Q . V .
18.583	1278.6105	856.61	.	. . Q . V .
18.667	1284.5020	855.45	.	. . Q . V .
18.750	1290.3645	851.24	.	. . Q . V .
18.833	1296.1793	844.31	.	. . Q . V .
18.917	1301.9497	837.87	.	. . Q . V .
19.000	1307.6602	829.16	.	. . Q . V .

TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
 (Note: 100% of Peak Flow Rate estimate assumed to have  
 an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1145.0
10%	1145.0
20%	330.0
30%	160.0
40%	70.0
50%	55.0
60%	40.0
70%	30.0
80%	25.0
90%	15.0

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 352.00 TO NODE 353.00 IS CODE = 5.2  
 -----  
 >>>>MODEL CHANNEL ROUTING OF STREAM #1 BY THE CONVEX METHOD<<<<<  
 =====

THE MODIFIED C-ROUTING COEFFICIENT IS ESTIMATED IN ORDER  
 TO ROUTE THE STREAM 1 INFLOW HYDROGRAPH BY 5-MINUTE  
 INTERVALS(Reference: the National Engineering Handbook,  
 Hydrology, Chapter 17, page 17-52, August,1972,  
 U.S. Department of Commerce).

ASSUMED REGULAR CHANNEL INFORMATION:  
 BASEWIDTH(FT) = 12.00 CHANNEL Z = 2.00  
 UPSTREAM ELEVATION(FT) = 1655.00  
 DOWNSTREAM ELEVATION(FT) = 1625.00  
 CHANNEL LENGTH(FT) = 1454.79 MANNING'S FACTOR = 0.015  
 CONSTANT LOSS RATE(CFS) = 0.00

CHANNEL ROUTING COEFFICIENT ESTIMATED:  
 MAXIMUM INFLOW(CFS) = 3946.80  
 AVERAGE FLOWRATE IN EXCESS OF 50% MAXIMUM INFLOW = 2970.53  
 CHANNEL NORMAL VELOCITY FOR Q = 2970.53 CFS = 29.74 FPS  
 ESTIMATED CHANNEL ROUTING COEFFICIENT = 0.946

MODIFIED CHANNEL ROUTING COEFFICIENT FOR 5-MINUTE  
 UNIT INTERVALS IS CSTAR = 1.000

CONVEX METHOD CHANNEL ROUTING RESULTS:

MODEL TIME (HRS)	INFLOW (STREAM 1) (CFS)	ROUTED FLOW (CFS)	OUTFLOW LESS
			LOSS (STREAM 1) (CFS)
16.000	1472.17	1460.38	1460.38
16.083	1726.17	1687.00	1687.00
16.167	2166.74	2098.80	2098.80
16.250	2738.49	2650.31	2650.31
16.333	3338.12	3245.65	3245.65
16.417	3755.13	3690.82	3690.82
16.500	3946.80	3917.24	3917.24
16.583	3775.47	3801.90	3801.90
16.667	3358.88	3423.13	3423.13
16.750	2881.42	2955.05	2955.05
16.833	2500.93	2559.61	2559.61
16.917	2210.22	2255.05	2255.05
17.000	2003.67	2035.53	2035.53
17.083	1836.48	1862.27	1862.27
17.167	1695.29	1717.06	1717.06
17.250	1565.59	1585.59	1585.59
17.333	1459.82	1476.13	1476.13
17.417	1367.32	1381.59	1381.59
17.500	1283.59	1296.50	1296.50
17.583	1203.65	1215.98	1215.98
17.667	1133.53	1144.34	1144.34
17.750	1069.52	1079.39	1079.39



17.833	1011.94	1020.82	1020.82
17.917	966.31	973.34	973.34
18.000	928.09	933.99	933.99
18.083	895.47	900.50	900.50
18.167	873.06	876.52	876.52
18.250	857.35	859.77	859.77
18.333	848.09	849.51	849.51
18.417	847.96	847.98	847.98
18.500	853.90	852.99	852.99
18.583	856.61	856.19	856.19
18.667	855.45	855.63	855.63
18.750	851.24	851.89	851.89
18.833	844.31	845.38	845.38
18.917	837.87	838.86	838.86
19.000	829.16	830.50	830.50

PROCESS SUMMARY OF STORAGE:

INFLOW VOLUME = 1600.928 AF  
 OUTFLOW VOLUME = 1600.928 AF  
 LOSS VOLUME = 0.000 AF

\*\*\*\*\*

FLOW PROCESS FROM NODE 353.00 TO NODE 353.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<

(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERSHED AREA = 50.170 ACRES  
 BASEFLOW = 0.000 CFS/SQUARE-MILE  
 \*USER ENTERED "LAG" TIME = 0.100 HOURS  
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.  
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)  
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.  
 VALLEY (DEVELOPED):  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.433  
 FOOTHILL "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 MOUNTAIN "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 VALLEY (UNDEVELOPED) / DESERT:  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.567  
 DESERT (UNDEVELOPED) "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 MAXIMUM WATERSHED LOSS RATE (INCH/HOUR) = 0.322  
 LOW LOSS FRACTION = 0.306  
 \*HYDROGRAPH MODEL #1 SPECIFIED\*

SPECIFIED PEAK 5-MINUTES RAINFALL (INCH) = 0.45  
 SPECIFIED PEAK 30-MINUTES RAINFALL (INCH) = 0.92  
 SPECIFIED PEAK 1-HOUR RAINFALL (INCH) = 1.22  
 SPECIFIED PEAK 3-HOUR RAINFALL (INCH) = 2.04  
 SPECIFIED PEAK 6-HOUR RAINFALL (INCH) = 2.83  
 SPECIFIED PEAK 24-HOUR RAINFALL (INCH) = 6.97

\*USER SPECIFIED PRECIPITATION DEPTH-AREA REDUCTION FACTORS:  
 5-MINUTE FACTOR = 0.820  
 30-MINUTE FACTOR = 0.820  
 1-HOUR FACTOR = 0.820

3-HOUR FACTOR = 0.973  
 6-HOUR FACTOR = 0.987  
 24-HOUR FACTOR = 0.992

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES  
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 83.333

RUNOFF HYDROGRAPH LISTING LIMITS:  
 MODEL TIME (HOURS) FOR BEGINNING OF RESULTS = 16.00  
 MODEL TIME (HOURS) FOR END OF RESULTS = 19.00

UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES (CFS)
1	13.170	79.908
2	62.562	299.685
3	85.799	140.986
4	92.075	38.078
5	94.971	17.573
6	96.847	11.385
7	98.101	7.603
8	98.891	4.797
9	99.262	2.251
10	99.597	2.031
11	99.839	1.468
12	99.960	0.734
13	100.000	0.245

TOTAL SOIL-LOSS VOLUME (ACRE-FEET) = 8.4464  
 TOTAL STORM RUNOFF VOLUME (ACRE-FEET) = 20.4475

24 - HOUR STORM  
 RUNOFF HYDROGRAPH

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)  
 (Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	50.0	100.0	150.0	200.0
16.000	12.3360	33.69	.	Q	.	V	.
16.083	12.7910	66.07	.	.	Q	.	V
16.167	13.6395	123.20	.	.	.	Q	V
16.250	14.1286	71.02	.	.	Q	.	V
16.333	14.3704	35.12	.	Q	.	.	V
16.417	14.5459	25.48	.	Q	.	.	V

16.500	14.7135	24.34	.	Q	.	.	V	.
16.583	14.8705	22.80	.	Q	.	.	V	.
16.667	15.0135	20.77	.	Q	.	.	V	.
16.750	15.1441	18.96	.	Q	.	.	V	.
16.833	15.2679	17.97	.	Q	.	.	V	.
16.917	15.3850	17.01	.	Q	.	.	V	.
17.000	15.4958	16.10	.	Q	.	.	V	.
17.083	15.5987	14.94	.	Q	.	.	V	.
17.167	15.6871	12.83	.	Q	.	.	V	.
17.250	15.7673	11.65	.	Q	.	.	V	.
17.333	15.8433	11.03	.	Q	.	.	V	.
17.417	15.9159	10.55	.	Q	.	.	V	.
17.500	15.9858	10.15	.	Q	.	.	V	.
17.583	16.0532	9.79	.	Q	.	.	V	.
17.667	16.1185	9.48	.	Q	.	.	V	.
17.750	16.1819	9.20	.	Q	.	.	V	.
17.833	16.2435	8.95	.	Q	.	.	V	.
17.917	16.3035	8.71	.	Q	.	.	V	.
18.000	16.3621	8.50	.	Q	.	.	V	.
18.083	16.4217	8.65	.	Q	.	.	V	.
18.167	16.4890	9.78	.	Q	.	.	V	.
18.250	16.5595	10.23	.	Q	.	.	V	.
18.333	16.6301	10.25	.	Q	.	.	V	.
18.417	16.7003	10.19	.	Q	.	.	V	.
18.500	16.7699	10.10	.	Q	.	.	V	.
18.583	16.8388	10.01	.	Q	.	.	V	.
18.667	16.9071	9.91	.	Q	.	.	V	.
18.750	16.9746	9.81	.	Q	.	.	V	.
18.833	17.0415	9.71	.	Q	.	.	V	.
18.917	17.1077	9.61	.	Q	.	.	V	.
19.000	17.1732	9.51	.	Q	.	.	V	.

-----  
TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
(Note: 100% of Peak Flow Rate estimate assumed to have  
an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
=====	=====
0%	1145.0
10%	185.0
20%	35.0
30%	15.0
40%	15.0
50%	15.0
60%	5.0
70%	5.0
80%	5.0
90%	5.0

\*\*\*\*\*  
FLOW PROCESS FROM NODE 353.00 TO NODE 353.00 IS CODE = 11

-----  
>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<  
=====

STREAM HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)

(Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	1000.0	2000.0	3000.0	4000.0
16.000	904.0137	1494.07	.	.	Q	.	V
16.083	916.0872	1753.06	.	.	Q	.	V
16.167	931.3902	2222.00	.	.	.	Q	.
16.250	950.1321	2721.33	.	.	.	V	Q
16.333	972.7269	3280.77	.	.	.	V	.
16.417	998.3213	3716.30	.	.	.	V	.
16.500	1025.4672	3941.58	.	.	.	V	.
16.583	1051.8080	3824.69	.	.	.	V	.
16.667	1075.5262	3443.90	.	.	.	V	.
16.750	1096.0084	2974.01	.	.	.	V	Q
16.833	1113.7604	2577.58	.	.	.	Q	V
16.917	1129.4082	2272.06	.	.	.	Q	V
17.000	1143.5378	2051.62	.	.	.	Q	V
17.083	1156.4662	1877.20	.	.	.	Q	V
17.167	1168.3800	1729.89	.	.	.	Q	V
17.250	1179.3802	1597.24	.	.	.	Q	V
17.333	1189.6224	1487.16	.	.	.	Q	V
17.417	1199.2102	1392.14	.	.	.	Q	V
17.500	1208.2092	1306.65	.	.	.	Q	V
17.583	1216.6511	1225.77	.	.	.	Q	V
17.667	1224.5975	1153.82	.	.	.	Q	V
17.750	1232.0947	1088.59	.	.	.	Q	V
17.833	1239.1868	1029.77	.	.	.	Q	V
17.917	1245.9502	982.06	.	.	.	Q	V
18.000	1252.4412	942.49	.	.	.	Q	V
18.083	1258.7025	909.15	.	.	.	Q	V
18.167	1264.8065	886.30	.	.	.	Q	V
18.250	1270.7983	870.01	.	.	.	Q	V
18.333	1276.7196	859.77	.	.	.	Q	V
18.417	1282.6299	858.17	.	.	.	Q	V
18.500	1288.5741	863.09	.	.	.	Q	V
18.583	1294.5397	866.21	.	.	.	Q	V
18.667	1300.5007	865.54	.	.	.	Q	V
18.750	1306.4353	861.70	.	.	.	Q	V
18.833	1312.3243	855.09	.	.	.	Q	V
18.917	1318.1678	848.47	.	.	.	Q	V
19.000	1323.9530	840.01	.	.	.	Q	V

-----  
TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
(Note: 100% of Peak Flow Rate estimate assumed to have  
an instantaneous time duration)

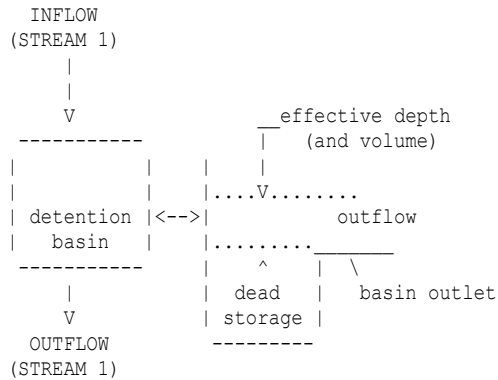
Percentile of Estimated Peak Flow Rate	Duration (minutes)
=====	=====
0%	1145.0
10%	1145.0
20%	355.0
30%	160.0
40%	75.0
50%	55.0
60%	40.0
70%	30.0
80%	25.0

90%

15.0

\*\*\*\*\*  
FLOW PROCESS FROM NODE 353.00 TO NODE 353.00 IS CODE = 3.1

>>>>FLOW-THROUGH DETENTION BASIN ROUTING MODEL APPLIED TO STREAM #1<<<<



ROUTE RUNOFF HYDROGRAPH FROM STREAM NUMBER 1 THROUGH A FLOW-THROUGH DETENTION BASIN

SPECIFIED BASIN CONDITIONS ARE AS FOLLOWS:

DEAD STORAGE (AF) = 0.000  
SPECIFIED DEAD STORAGE (AF) FILLED = 0.000  
SPECIFIED EFFECTIVE VOLUME (AF) FILLED ABOVE OUTLET = 0.000  
DETENTION BASIN CONSTANT LOSS RATE (CFS) = 0.00

BASIN DEPTH VERSUS OUTFLOW AND STORAGE INFORMATION:

INTERVAL NUMBER	DEPTH (FT)	OUTFLOW (CFS)	STORAGE (AF)
1	0.00	0.00	0.000
2	1.00	8.18	0.190
3	2.00	31.40	1.260
4	4.00	113.93	7.830
5	6.00	243.56	19.470
6	8.00	314.44	35.170
7	10.00	372.05	54.690
8	11.00	397.74	65.080
9	12.00	421.86	75.730
10	14.00	466.39	97.820
11	16.00	507.02	121.020
12	18.00	544.62	145.370
13	20.00	579.80	170.940
14	22.00	612.95	198.070
15	24.00	644.41	226.570
16	26.00	674.40	256.470
17	28.00	703.11	287.820
18	30.00	730.69	320.680

19 31.00 744.10 337.700  
20 32.00 757.27 355.120

=====

MODIFIED-PULS BASIN ROUTING MODEL RESULTS (5-MINUTE COMPUTATION INTERVALS):  
(Note: Computed EFFECTIVE DEPTH and VOLUME are estimated at the clock time;  
MEAN OUTFLOW is the average value during the unit interval.)

CLOCK TIME (HRS)	DEAD-STORAGE FILLED (AF)	INFLOW (CFS)	LOSS (CFS)	EFFECTIVE DEPTH (FT)	MEAN OUTFLOW (CFS)	EFFECTIVE VOLUME (AF)
16.083	0.000	1753.06	0.00	28.29	704.0	292.565
16.167	0.000	2222.00	0.00	28.92	711.5	302.968
16.250	0.000	2721.33	0.00	29.76	721.6	316.740
16.333	0.000	3280.77	0.00	30.80	734.4	334.277
16.417	0.000	3716.30	0.00	31.98	749.2	354.712
16.500	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
16.500	0.000	3941.58	0.00	33.23	765.2	376.588
16.583	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
16.583	0.000	3824.69	0.00	34.44	781.4	397.547
16.667	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
16.667	0.000	3443.90	0.00	35.48	796.2	415.781
16.750	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
16.750	0.000	2974.01	0.00	36.34	808.8	430.694
16.833	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
16.833	0.000	2577.58	0.00	37.03	819.0	442.805
16.917	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
16.917	0.000	2272.06	0.00	37.60	827.3	452.755
17.000	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.000	0.000	2051.62	0.00	38.09	834.3	461.139
17.083	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.083	0.000	1877.20	0.00	38.50	840.1	468.282
17.167	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.167	0.000	1729.89	0.00	38.85	845.1	474.375
17.250	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.250	0.000	1597.24	0.00	39.14	849.4	479.526
17.333	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.333	0.000	1487.16	0.00	39.39	853.0	483.893
17.417	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.417	0.000	1392.14	0.00	39.60	856.0	487.586
17.500	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.500	0.000	1306.65	0.00	39.78	858.6	490.671
17.583	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.583	0.000	1225.77	0.00	39.93	860.7	493.186
17.667	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.667	0.000	1153.82	0.00	40.04	862.4	495.193
17.750	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.750	0.000	1088.59	0.00	40.13	863.8	496.741
17.833	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.833	0.000	1029.77	0.00	40.20	864.8	497.878
17.917	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
17.917	0.000	982.06	0.00	40.24	865.5	498.680
18.000	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
18.000	0.000	942.49	0.00	40.27	866.0	499.207
18.083	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*					
18.083	0.000	909.15	0.00	40.29	866.3	499.502

18.167	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
18.167	0.000 886.30 0.00 40.30	866.5 499.639
18.250	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
18.250	0.000 870.01 0.00 40.30	866.5 499.663
18.333	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
18.333	0.000 859.77 0.00 40.29	866.5 499.616
18.417	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
18.417	0.000 858.17 0.00 40.29	866.5 499.559
18.500	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
18.500	0.000 863.09 0.00 40.29	866.5 499.536
18.583	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
18.583	0.000 866.21 0.00 40.29	866.5 499.534
18.667	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
18.667	0.000 865.54 0.00 40.29	866.4 499.528
18.750	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
18.750	0.000 861.70 0.00 40.29	866.4 499.495
18.833	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
18.833	0.000 855.09 0.00 40.28	866.4 499.417
18.917	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
18.917	0.000 848.47 0.00 40.28	866.3 499.294
19.000	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.000	0.000 840.01 0.00 40.27	866.2 499.114
19.083	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.083	0.000 827.65 0.00 40.25	866.0 498.850
19.167	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.167	0.000 813.93 0.00 40.23	865.8 498.493
19.250	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.250	0.000 802.28 0.00 40.21	865.5 498.057
19.333	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.333	0.000 793.42 0.00 40.18	865.1 497.563
19.417	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.417	0.000 785.42 0.00 40.15	864.8 497.017
19.500	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.500	0.000 777.60 0.00 40.11	864.3 496.420
19.583	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.583	0.000 769.93 0.00 40.07	863.9 495.773
19.667	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.667	0.000 762.49 0.00 40.03	863.3 495.078
19.750	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.750	0.000 755.25 0.00 39.99	862.8 494.338
19.833	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.833	0.000 748.18 0.00 39.95	862.2 493.552
19.917	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
19.917	0.000 741.17 0.00 39.90	861.6 492.723
20.000	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.000	0.000 733.18 0.00 39.85	861.0 491.843
20.083	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.083	0.000 725.30 0.00 39.80	860.3 490.913
20.167	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.167	0.000 718.73 0.00 39.74	859.6 489.943
20.250	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.250	0.000 712.53 0.00 39.68	858.8 488.936
20.333	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.333	0.000 705.86 0.00 39.62	858.0 487.888
20.417	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.417	0.000 697.94 0.00 39.56	857.2 486.791
20.500	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.500	0.000 689.19 0.00 39.49	856.4 485.639

20.583	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.583	0.000 680.82 0.00 39.42	855.5 484.436
20.667	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.667	0.000 674.84 0.00 39.35	854.6 483.198
20.750	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.750	0.000 669.70 0.00 39.28	853.6 481.932
20.833	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.833	0.000 664.71 0.00 39.21	852.7 480.637
20.917	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
20.917	0.000 659.81 0.00 39.13	851.7 479.316
21.000	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.000	0.000 655.07 0.00 39.05	850.7 477.969
21.083	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.083	0.000 650.46 0.00 38.97	849.6 476.597
21.167	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.167	0.000 645.96 0.00 38.89	848.6 475.202
21.250	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.250	0.000 641.56 0.00 38.81	847.5 473.784
21.333	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.333	0.000 637.28 0.00 38.73	846.4 472.343
21.417	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.417	0.000 633.16 0.00 38.65	845.3 470.882
21.500	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.500	0.000 629.19 0.00 38.56	844.2 469.401
21.583	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.583	0.000 625.38 0.00 38.47	843.1 467.901
21.667	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.667	0.000 621.68 0.00 38.39	842.0 466.384
21.750	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.750	0.000 618.08 0.00 38.30	840.8 464.850
21.833	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.833	0.000 614.57 0.00 38.21	839.6 463.300
21.917	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
21.917	0.000 611.15 0.00 38.12	838.5 461.735
22.000	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.000	0.000 607.80 0.00 38.03	837.3 460.154
22.083	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.083	0.000 604.50 0.00 37.94	836.1 458.559
22.167	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.167	0.000 601.27 0.00 37.85	834.9 456.951
22.250	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.250	0.000 598.11 0.00 37.75	833.6 455.329
22.333	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.333	0.000 595.02 0.00 37.66	832.4 453.694
22.417	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.417	0.000 591.98 0.00 37.56	831.2 452.046
22.500	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.500	0.000 588.97 0.00 37.47	829.9 450.387
22.583	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.583	0.000 585.99 0.00 37.37	828.7 448.716
22.667	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.667	0.000 583.07 0.00 37.28	827.4 447.033
22.750	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.750	0.000 580.20 0.00 37.18	826.1 445.339
22.833	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.833	0.000 577.39 0.00 37.08	824.8 443.635
22.917	*BASIN CAPACITY EXCEEDED: BASIN DATA IS	EXTRAPOLATED*
22.917	0.000 574.63 0.00 36.98	823.5 441.921

23.000	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.000	0.000 571.92 0.00 36.88 822.2 440.197
23.083	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.083	0.000 569.26 0.00 36.78 820.9 438.464
23.167	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.167	0.000 566.65 0.00 36.68 819.6 436.722
23.250	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.250	0.000 564.09 0.00 36.58 818.3 434.971
23.333	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.333	0.000 561.57 0.00 36.48 817.0 433.212
23.417	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.417	0.000 559.10 0.00 36.38 815.6 431.445
23.500	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.500	0.000 556.67 0.00 36.28 814.3 429.671
23.583	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.583	0.000 554.28 0.00 36.18 813.0 427.889
23.667	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.667	0.000 551.93 0.00 36.07 811.6 426.101
23.750	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.750	0.000 549.62 0.00 35.97 810.3 424.306
23.833	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.833	0.000 547.35 0.00 35.87 808.9 422.504
23.917	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
23.917	0.000 545.11 0.00 35.76 807.5 420.697
24.000	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.083	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.167	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.250	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.333	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.417	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.500	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.583	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.667	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.750	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.833	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
24.917	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
25.000	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
25.083	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
25.167	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
25.250	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*
25.333	*BASIN CAPACITY EXCEEDED: BASIN DATA IS EXTRAPOLATED*

-----  
PROCESS SUMMARY OF STORAGE:

INFLOW VOLUME = 1621.376 AF  
BASIN STORAGE = 0.000 AF (WITH 0.000 AF INITIALLY FILLED)  
OUTFLOW VOLUME = 1621.380 AF  
LOSS VOLUME = 0.000 AF  
=====

END OF FLOODSCx ROUTING ANALYSIS

\*\*\*\*\*

FLOOD ROUTING ANALYSIS  
USING COUNTY HYDROLOGY MANUAL OF SAN BERNARDINO(1986)  
(c) Copyright 1989-2011 Advanced Engineering Software (aes)  
Ver. 18.0 Release Date: 05/01/2011 License ID 1224

Analysis prepared by:

San Bernardino County  
Public Works Department

Water Resources Division

\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* Opal Hydrology W/o Basin \*  
\* Opal Feasibility Study \*  
\* Node 20353 AMC-III \*  
\*\*\*\*\*

FILE NAME: OP353A.DAT  
TIME/DATE OF STUDY: 13:23 09/12/2013

\*\*\*\*\*  
FLOW PROCESS FROM NODE 250.00 TO NODE 250.00 IS CODE = 1  
\*\*\*\*\*

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<

(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERSHED AREA = 2221.150 ACRES  
BASEFLOW = 0.000 CFS/SQUARE-MILE  
\*USER ENTERED "LAG" TIME = 0.490 HOURS  
CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.  
THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)  
MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.  
VALLEY(DEVELOPED):  
"S"-CURVE PERCENTAGE(DECIMAL NOTATION) = 0.384  
FOOTHILL "S"-CURVE PERCENTAGE(DECIMAL NOTATION) = 0.000  
MOUNTAIN "S"-CURVE PERCENTAGE(DECIMAL NOTATION) = 0.000  
VALLEY(UNDEVELOPED)/DESERT:  
"S"-CURVE PERCENTAGE(DECIMAL NOTATION) = 0.616  
DESERT(UNDEVELOPED) "S"-CURVE PERCENTAGE(DECIMAL NOTATION) = 0.000  
MAXIMUM WATERSHED LOSS RATE (INCH/HOUR) = 0.350  
LOW LOSS FRACTION = 0.310  
\*HYDROGRAPH MODEL #1 SPECIFIED\*

SPECIFIED PEAK 5-MINUTES RAINFALL(INCH)= 0.46  
SPECIFIED PEAK 30-MINUTES RAINFALL(INCH)= 0.94  
SPECIFIED PEAK 1-HOUR RAINFALL(INCH) = 1.24  
SPECIFIED PEAK 3-HOUR RAINFALL(INCH) = 2.13  
SPECIFIED PEAK 6-HOUR RAINFALL(INCH) = 3.01  
SPECIFIED PEAK 24-HOUR RAINFALL(INCH) = 7.33

\*USER SPECIFIED PRECIPITATION DEPTH-AREA REDUCTION FACTORS:

5-MINUTE FACTOR = 0.820  
30-MINUTE FACTOR = 0.820  
1-HOUR FACTOR = 0.820  
3-HOUR FACTOR = 0.973  
6-HOUR FACTOR = 0.987  
24-HOUR FACTOR = 0.992

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES  
UNIT INTERVAL PERCENTAGE OF LAG-TIME = 17.007

RUNOFF HYDROGRAPH LISTING LIMITS:  
MODEL TIME(HOURS) FOR BEGINNING OF RESULTS = 16.00  
MODEL TIME(HOURS) FOR END OF RESULTS = 19.00

UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES (CFS)
1	1.264	339.471
2	4.631	904.392
3	10.785	1653.291
4	20.280	2550.490
5	31.798	3093.866
6	44.681	3460.766
7	56.611	3204.613
8	65.772	2460.882
9	72.515	1811.287
10	77.424	1318.574
11	81.028	968.023
12	83.809	747.209
13	86.000	588.511
14	87.702	457.146
15	88.983	344.116
16	90.061	289.541
17	91.003	253.192
18	91.867	232.069
19	92.618	201.551
20	93.356	198.299
21	94.000	172.960
22	94.489	131.369
23	94.972	129.678
24	95.384	110.695
25	95.772	104.231
26	96.159	104.063
27	96.498	90.910
28	96.806	82.778
29	97.114	82.722
30	97.421	82.589
31	97.664	65.255
32	97.867	54.654
33	98.071	54.615
34	98.274	54.654
35	98.478	54.615

36	98.681	54.617
37	98.811	34.926
38	98.886	20.219
39	98.961	20.183
40	99.037	20.217
41	99.112	20.183
42	99.187	20.219
43	99.262	20.183
44	99.337	20.181
45	99.413	20.219
46	99.488	20.181
47	99.563	20.183
48	99.638	20.181
49	99.713	20.181
50	99.788	20.183
51	99.863	20.181
52	99.938	20.183
53	100.000	16.537

-----  
TOTAL SOIL-LOSS VOLUME (ACRE-FEET) = 399.4406  
TOTAL STORM RUNOFF VOLUME (ACRE-FEET) = 945.8401  
-----

=====

2 4 - H O U R   S T O R M  
R U N O F F   H Y D R O G R A P H

=====

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)  
(Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	575.0	1150.0	1725.0	2300.0
16.000	531.0939	870.28	.	.	Q	. V	.
16.083	538.3464	1053.07	.	.	Q	. V	.
16.167	547.4697	1324.70	.	.	.	Q	.
16.250	558.8416	1651.20	.	.	.	V	Q
16.333	572.5420	1989.30	.	.	.	V	Q
16.417	587.5893	2184.86	.	.	.	V	Q
16.500	603.2255	2270.37	.	.	.	V	Q
16.583	617.9086	2132.00	.	.	.	V	Q
16.667	630.5999	1842.77	.	.	.	V	Q
16.750	641.5264	1586.54	.	.	.	Q	.
16.833	651.1153	1392.30	.	.	.	Q	V
16.917	659.7304	1250.91	.	.	.	Q	V
17.000	667.6461	1149.35	.	.	.	Q	V
17.083	674.9822	1065.21	.	.	.	Q	V
17.167	681.7884	988.26	.	.	.	Q	V
17.250	688.0996	916.39	.	.	.	Q	V
17.333	694.0256	860.45	.	.	.	Q	V
17.417	699.6032	809.88	.	.	.	Q	V
17.500	704.8595	763.21	.	.	.	Q	V
17.583	709.7971	716.93	.	.	.	Q	V
17.667	714.4987	682.67	.	.	.	Q	V

17.750	718.9498	646.31	.	.Q	.	V	.
17.833	723.1519	610.14	.	Q	.	V	.
17.917	727.2042	588.38	.	Q	.	V	.
18.000	731.0951	564.97	.	Q	.	V	.
18.083	734.8704	548.16	.	Q	.	.V	.
18.167	738.5620	536.02	.	Q	.	.V	.
18.250	742.1741	524.48	.	Q	.	.V	.
18.333	745.7465	518.72	.	Q	.	.V	.
18.417	749.3156	518.22	.	Q	.	.V	.
18.500	752.8924	519.36	.	Q	.	.V	.
18.583	756.4420	515.40	.	Q	.	.V	.
18.667	759.9651	511.57	.	Q	.	. V	.
18.750	763.4725	509.27	.	Q	.	. V	.
18.833	766.9568	505.91	.	Q	.	. V	.
18.917	770.4078	501.09	.	Q	.	. V	.
19.000	773.8166	494.95	.	Q	.	. V	.

-----  
TIME DURATION (minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
(Note: 100% of Peak Flow Rate estimate assumed to have  
an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1145.0
10%	1145.0
20%	400.0
30%	175.0
40%	75.0
50%	55.0
60%	40.0
70%	30.0
80%	25.0
90%	15.0

\*\*\*\*\*  
FLOW PROCESS FROM NODE 250.00 TO NODE 274.00 IS CODE = 5.2  
-----

>>>>MODEL CHANNEL ROUTING OF STREAM #1 BY THE CONVEX METHOD<<<<<

-----  
THE MODIFIED C-ROUTING COEFFICIENT IS ESTIMATED IN ORDER  
TO ROUTE THE STREAM 1 INFLOW HYDROGRAPH BY 5-MINUTE  
INTERVALS (Reference: the National Engineering Handbook,  
Hydrology, Chapter 17, page 17-52, August, 1972,  
U.S. Department of Commerce).

ASSUMED REGULAR CHANNEL INFORMATION:  
BASEWIDTH (FT) = 30.00      CHANNEL Z = 2.50  
UPSTREAM ELEVATION (FT) = 1733.00  
DOWNSTREAM ELEVATION (FT) = 1670.00  
CHANNEL LENGTH (FT) = 2379.03      MANNING'S FACTOR = 0.035  
CONSTANT LOSS RATE (CFS) = 0.00

CHANNEL ROUTING COEFFICIENT ESTIMATED:

MAXIMUM INFLOW(CFS) = 2270.37  
 AVERAGE FLOWRATE IN EXCESS OF 50% MAXIMUM INFLOW = 1706.76  
 CHANNEL NORMAL VELOCITY FOR Q = 1706.76 CFS = 13.28 FPS  
 ESTIMATED CHANNEL ROUTING COEFFICIENT = 0.887

MODIFIED CHANNEL ROUTING COEFFICIENT FOR 5-MINUTE  
 UNIT INTERVALS IS CSTAR = 0.969

CONVEX METHOD CHANNEL ROUTING RESULTS:

MODEL TIME (HRS)	INFLOW (STREAM 1) (CFS)	ROUTED FLOW (CFS)	OUTFLOW LESS
			LOSS (STREAM 1) (CFS)
16.000	870.28	842.01	842.01
16.083	1053.07	952.73	952.73
16.167	1324.70	1173.77	1173.77
16.250	1651.20	1468.83	1468.83
16.333	1989.30	1799.64	1799.64
16.417	2184.86	2072.53	2072.53
16.500	2270.37	2220.34	2220.34
16.583	2132.00	2205.72	2205.72
16.667	1842.77	2002.44	2002.44
16.750	1586.54	1730.94	1730.94
16.833	1392.30	1502.50	1502.50
16.917	1250.91	1331.29	1331.29
17.000	1149.35	1207.12	1207.12
17.083	1065.21	1112.80	1112.80
17.167	988.26	1031.61	1031.61
17.250	916.39	956.85	956.85
17.333	860.45	892.15	892.15
17.417	809.88	838.38	838.38
17.500	763.21	789.49	789.49
17.583	716.93	742.93	742.93
17.667	682.67	702.13	702.13
17.750	646.31	666.70	666.70
17.833	610.14	630.46	630.46
17.917	588.38	600.86	600.86
18.000	564.97	578.10	578.10
18.083	548.16	557.72	557.72
18.167	536.02	542.93	542.93
18.250	524.48	530.98	530.98
18.333	518.72	522.06	522.06
18.417	518.22	518.60	518.60
18.500	519.36	518.75	518.75
18.583	515.40	517.53	517.53
18.667	511.57	513.72	513.72
18.750	509.27	510.59	510.59
18.833	505.91	507.78	507.78
18.917	501.09	503.78	503.78
19.000	494.95	498.38	498.38

PROCESS SUMMARY OF STORAGE:

INFLOW VOLUME = 945.840 AF  
 OUTFLOW VOLUME = 945.840 AF  
 LOSS VOLUME = 0.000 AF

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 274.00 TO NODE 274.00 IS CODE = 1  
 -----  
 >>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<  
 -----

(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERSHED AREA = 880.780 ACRES  
 BASEFLOW = 0.000 CFS/SQUARE-MILE  
 \*USER ENTERED "LAG" TIME = 0.440 HOURS  
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.  
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)  
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.  
 VALLEY (DEVELOPED):  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.266  
 FOOTHILL "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 MOUNTAIN "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 VALLEY (UNDEVELOPED) / DESERT:  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.730  
 DESERT (UNDEVELOPED) "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.004  
 MAXIMUM WATERSHED LOSS RATE (INCH/HOUR) = 0.357  
 LOW LOSS FRACTION = 0.338  
 \*HYDROGRAPH MODEL #1 SPECIFIED\*

SPECIFIED PEAK 5-MINUTES RAINFALL (INCH) = 0.41  
 SPECIFIED PEAK 30-MINUTES RAINFALL (INCH) = 0.85  
 SPECIFIED PEAK 1-HOUR RAINFALL (INCH) = 1.12  
 SPECIFIED PEAK 3-HOUR RAINFALL (INCH) = 1.82  
 SPECIFIED PEAK 6-HOUR RAINFALL (INCH) = 2.47  
 SPECIFIED PEAK 24-HOUR RAINFALL (INCH) = 6.29

\*USER SPECIFIED PRECIPITATION DEPTH-AREA REDUCTION FACTORS:  
 5-MINUTE FACTOR = 0.820  
 30-MINUTE FACTOR = 0.820  
 1-HOUR FACTOR = 0.820  
 3-HOUR FACTOR = 0.973  
 6-HOUR FACTOR = 0.987  
 24-HOUR FACTOR = 0.992

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES  
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 18.939

RUNOFF HYDROGRAPH LISTING LIMITS:  
 MODEL TIME (HOURS) FOR BEGINNING OF RESULTS = 16.00  
 MODEL TIME (HOURS) FOR END OF RESULTS = 19.00

UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES (CFS)
1	1.485	158.232
2	5.716	450.656



3	13.433	821.952
4	24.937	1225.442
5	38.586	1453.903
6	52.403	1471.774
7	62.830	1110.684
8	70.231	788.306
9	75.496	560.876
10	79.347	410.158
11	82.289	313.405
12	84.610	247.203
13	86.461	197.174
14	87.889	152.072
15	89.139	133.241
16	90.249	118.206
17	91.205	101.799
18	92.115	96.919
19	92.916	85.333
20	93.564	69.004
21	94.185	66.192
22	94.710	55.879
23	95.224	54.820
24	95.708	51.506
25	96.119	43.842
26	96.528	43.528
27	96.934	43.276
28	97.251	33.750
29	97.521	28.725
30	97.790	28.695
31	98.060	28.692
32	98.329	28.676
33	98.552	23.781
34	98.658	11.259
35	98.758	10.666
36	98.858	10.681
37	98.958	10.651
38	99.058	10.681
39	99.158	10.649
40	99.258	10.633
41	99.358	10.586
42	99.457	10.569
43	99.556	10.569
44	99.655	10.569
45	99.755	10.569
46	99.854	10.569
47	99.953	10.568
48	100.000	5.013

-----  
TOTAL SOIL-LOSS VOLUME (ACRE-FEET) = 148.0885  
TOTAL STORM RUNOFF VOLUME (ACRE-FEET) = 309.6898  
-----

2 4 - H O U R S T O R M  
R U N O F F H Y D R O G R A P H

=====

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)  
(Note: Time indicated is at END of Each Unit Intervals)

-----

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	225.0	450.0	675.0	900.0
16.000	173.3142	298.76	.	. Q	. V	.	.
16.083	175.9199	378.35	.	.	Q	. V	.
16.167	179.3562	498.95	.	.	.	QV	.
16.250	183.7354	635.86	.	.	.	V	Q.
16.333	188.9945	763.63	.	.	.	V	. Q
16.417	194.6603	822.67	.	.	.	V	. Q
16.500	200.1864	802.39	.	.	.	V	. Q
16.583	204.8531	677.62	.	.	.	V	Q
16.667	208.7421	564.68	.	.	.	QV	.
16.750	212.0680	482.93	.	.	.Q	V	.
16.833	215.0086	426.97	.	.	. Q	V	.
16.917	217.6713	386.62	.	.	. Q	V	.
17.000	220.1173	355.16	.	.	. Q	V	.
17.083	222.3774	328.17	.	.	. Q	V	.
17.167	224.4641	302.99	.	.	. Q	V	.
17.250	226.4204	284.05	.	. Q	.	V	.
17.333	228.2479	265.35	.	.Q	.	V	.
17.417	229.9444	246.34	.	Q	.	V	.
17.500	231.5312	230.41	.	Q	.	V	.
17.583	233.0117	214.96	.	Q.	.	V	.
17.667	234.3937	200.67	.	Q.	.	V	.
17.750	235.7108	191.25	.	Q.	.	V	.
17.833	236.9604	181.43	.	Q.	.	V	.
17.917	238.1634	174.68	.	Q.	.	V	.
18.000	239.3191	167.80	.	Q.	.	V	.
18.083	240.4304	161.36	.	Q.	.	.V	.
18.167	241.5231	158.66	.	Q.	.	.V	.
18.250	242.6107	157.92	.	Q.	.	.V	.
18.333	243.6933	157.20	.	Q.	.	.V	.
18.417	244.7895	159.16	.	Q.	.	.V	.
18.500	245.9107	162.80	.	Q.	.	.V	.
18.583	247.0465	164.92	.	Q.	.	.V	.
18.667	248.1851	165.32	.	Q.	.	.V	.
18.750	249.3097	163.29	.	Q.	.	.V	.
18.833	250.4058	159.15	.	Q.	.	.V	.
18.917	251.4931	157.88	.	Q.	.	.V	.
19.000	252.5728	156.76	.	Q.	.	.V	.

-----

TIME DURATION (minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
(Note: 100% of Peak Flow Rate estimate assumed to have  
an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1145.0
10%	1145.0
20%	230.0
30%	120.0
40%	60.0
50%	45.0

60% 35.0  
 70% 25.0  
 80% 20.0  
 90% 15.0

\*\*\*\*\*

FLOW PROCESS FROM NODE 274.00 TO NODE 274.00 IS CODE = 11

>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<

STREAM HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)  
 (Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	775.0	1550.0	2325.0	3100.0
16.000	701.0480	1140.78	.	.	Q	.	V
16.083	710.2152	1331.08	.	.	Q	.	V
16.167	721.7353	1672.71	.	.	.	Q	V
16.250	736.2304	2104.69	.	.	.	V	Q
16.333	753.8838	2563.27	.	.	.	V	Q
16.417	773.8232	2895.20	.	.	.	V	Q
16.500	794.6408	3022.72	.	.	.	V	Q
16.583	814.4985	2883.34	.	.	.	V	Q
16.667	832.1784	2567.12	.	.	.	V	Q
16.750	847.4254	2213.87	.	.	.	V	Q
16.833	860.7138	1929.47	.	.	.	Q	V
16.917	872.5451	1717.91	.	.	.	Q	V
17.000	883.3046	1562.28	.	.	.	Q	V
17.083	893.2286	1440.97	.	.	.	Q	V
17.167	902.4201	1334.60	.	.	.	Q	V
17.250	910.9663	1240.91	.	.	.	Q	V
17.333	918.9380	1157.50	.	.	.	Q	V
17.417	926.4086	1084.72	.	.	.	Q	V
17.500	933.4327	1019.90	.	.	.	Q	V
17.583	940.0297	957.89	.	.	.	Q	V
17.667	946.2473	902.79	.	.	.	Q	V
17.750	952.1561	857.96	.	.	.	Q	V
17.833	957.7477	811.89	.	.	.	Q	V
17.917	963.0889	775.54	.	.	.	Q	V
18.000	968.2260	745.90	.	.	.	Q	V
18.083	973.1782	719.07	.	.	.	Q	V
18.167	978.0101	701.59	.	.	.	Q	V
18.250	982.7545	688.89	.	.	.	Q	V
18.333	987.4326	679.26	.	.	.	Q	V
18.417	992.1003	677.76	.	.	.	Q	V
18.500	996.7943	681.56	.	.	.	Q	V
18.583	1001.4943	682.45	.	.	.	Q	V
18.667	1006.1709	679.04	.	.	.	Q	V
18.750	1010.8120	673.88	.	.	.	Q	V
18.833	1015.4052	666.93	.	.	.	Q	V
18.917	1019.9620	661.66	.	.	.	Q	V
19.000	1024.4740	655.14	.	.	.	Q	V

TIME DURATION (minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
 (Note: 100% of Peak Flow Rate estimate assumed to have an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1145.0
10%	1145.0
20%	390.0
30%	165.0
40%	75.0
50%	55.0
60%	40.0
70%	30.0
80%	25.0
90%	15.0

\*\*\*\*\*

FLOW PROCESS FROM NODE 274.00 TO NODE 352.00 IS CODE = 5.2

>>>>MODEL CHANNEL ROUTING OF STREAM #1 BY THE CONVEX METHOD<<<<

THE MODIFIED C-ROUTING COEFFICIENT IS ESTIMATED IN ORDER TO ROUTE THE STREAM 1 INFLOW HYDROGRAPH BY 5-MINUTE INTERVALS (Reference: the National Engineering Handbook, Hydrology, Chapter 17, page 17-52, August, 1972, U.S. Department of Commerce).

ASSUMED REGULAR CHANNEL INFORMATION:

BASEWIDTH (FT) = 10.00 CHANNEL Z = 2.00  
 UPSTREAM ELEVATION (FT) = 1670.00  
 DOWNSTREAM ELEVATION (FT) = 1655.00  
 CHANNEL LENGTH (FT) = 623.43 MANNING'S FACTOR = 0.015  
 CONSTANT LOSS RATE (CFS) = 0.00

CHANNEL ROUTING COEFFICIENT ESTIMATED:

MAXIMUM INFLOW (CFS) = 3022.72  
 AVERAGE FLOWRATE IN EXCESS OF 50% MAXIMUM INFLOW = 2284.78  
 CHANNEL NORMAL VELOCITY FOR Q = 2284.78 CFS = 29.68 FPS  
 ESTIMATED CHANNEL ROUTING COEFFICIENT = 0.946

MODIFIED CHANNEL ROUTING COEFFICIENT FOR 5-MINUTE UNIT INTERVALS IS CSTAR = 1.000

CONVEX METHOD CHANNEL ROUTING RESULTS:

MODEL TIME (HRS)	INFLOW (STREAM 1) (CFS)	ROUTED FLOW (CFS)	OUTFLOW LESS LOSS (STREAM 1) (CFS)	
			ROUTED FLOW (CFS)	LOSS (CFS)
16.000	1140.78	1137.00	1137.00	
16.083	1331.08	1318.48	1318.48	
16.167	1672.71	1650.09	1650.09	
16.250	2104.69	2076.09	2076.09	
16.333	2563.27	2532.91	2532.91	
16.417	2895.20	2873.22	2873.22	

16.500	3022.72	3014.28	3014.28
16.583	2883.34	2892.57	2892.57
16.667	2567.12	2588.06	2588.06
16.750	2213.87	2237.26	2237.26
16.833	1929.47	1948.30	1948.30
16.917	1717.91	1731.92	1731.92
17.000	1562.28	1572.58	1572.58
17.083	1440.97	1449.00	1449.00
17.167	1334.60	1341.65	1341.65
17.250	1240.91	1247.11	1247.11
17.333	1157.50	1163.02	1163.02
17.417	1084.72	1089.54	1089.54
17.500	1019.90	1024.20	1024.20
17.583	957.89	962.00	962.00
17.667	902.79	906.44	906.44
17.750	857.96	860.92	860.92
17.833	811.89	814.94	814.94
17.917	775.54	777.95	777.95
18.000	745.90	747.86	747.86
18.083	719.07	720.85	720.85
18.167	701.59	702.75	702.75
18.250	688.89	689.73	689.73
18.333	679.26	679.89	679.89
18.417	677.76	677.86	677.86
18.500	681.56	681.30	681.30
18.583	682.45	682.39	682.39
18.667	679.04	679.27	679.27
18.750	673.88	674.22	674.22
18.833	666.93	667.39	667.39
18.917	661.66	662.01	662.01
19.000	655.14	655.57	655.57

=====

PROCESS SUMMARY OF STORAGE:

INFLOW VOLUME = 1255.530 AF  
 OUTFLOW VOLUME = 1255.529 AF  
 LOSS VOLUME = 0.000 AF

\*\*\*\*\*

FLOW PROCESS FROM NODE 352.00 TO NODE 352.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<

=====

(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERSHED AREA = 877.380 ACRES  
 BASEFLOW = 0.000 CFS/SQUARE-MILE  
 \*USER ENTERED "LAG" TIME = 0.480 HOURS  
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.  
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)  
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.  
 VALLEY (DEVELOPED):  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.610  
 FOOTHILL "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 MOUNTAIN "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 VALLEY (UNDEVELOPED) / DESERT:  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.390

DESERT (UNDEVELOPED) "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 MAXIMUM WATERSHED LOSS RATE (INCH/HOUR) = 0.330  
 LOW LOSS FRACTION = 0.311  
 \*HYDROGRAPH MODEL #1 SPECIFIED\*

SPECIFIED PEAK 5-MINUTES RAINFALL (INCH) = 0.46  
 SPECIFIED PEAK 30-MINUTES RAINFALL (INCH) = 0.94  
 SPECIFIED PEAK 1-HOUR RAINFALL (INCH) = 1.24  
 SPECIFIED PEAK 3-HOUR RAINFALL (INCH) = 2.02  
 SPECIFIED PEAK 6-HOUR RAINFALL (INCH) = 2.75  
 SPECIFIED PEAK 24-HOUR RAINFALL (INCH) = 6.77

\*USER SPECIFIED PRECIPITATION DEPTH-AREA REDUCTION FACTORS:  
 5-MINUTE FACTOR = 0.820  
 30-MINUTE FACTOR = 0.820  
 1-HOUR FACTOR = 0.820  
 3-HOUR FACTOR = 0.973  
 6-HOUR FACTOR = 0.987  
 24-HOUR FACTOR = 0.992

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES  
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 17.361

RUNOFF HYDROGRAPH LISTING LIMITS:  
 MODEL TIME (HOURS) FOR BEGINNING OF RESULTS = 16.00  
 MODEL TIME (HOURS) FOR END OF RESULTS = 19.00

=====

UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES (CFS)
1	1.182	125.446
2	4.354	336.580
3	10.707	674.107
4	20.574	1046.928
5	32.294	1243.622
6	45.868	1440.305
7	58.683	1359.796
8	69.021	1096.918
9	76.382	781.054
10	81.721	566.459
11	85.386	388.965
12	88.182	296.636
13	90.180	212.057
14	91.687	159.908
15	92.620	99.009
16	93.406	83.359
17	94.116	75.360
18	94.771	69.478
19	95.365	63.026
20	95.959	63.067
21	96.334	39.750
22	96.649	33.412

23	96.945	31.420
24	97.198	26.793
25	97.448	26.610
26	97.688	25.428
27	97.890	21.453
28	98.089	21.127
29	98.288	21.135
30	98.469	19.163
31	98.603	14.199
32	98.734	13.948
33	98.866	13.948
34	98.997	13.957
35	99.129	13.939
36	99.233	11.099
37	99.283	5.316
38	99.332	5.168
39	99.381	5.158
40	99.429	5.149
41	99.478	5.149
42	99.526	5.167
43	99.575	5.140
44	99.624	5.177
45	99.672	5.140
46	99.721	5.140
47	99.769	5.140
48	99.817	5.139
49	99.866	5.140
50	99.914	5.140
51	99.963	5.140
52	100.000	3.952

-----  
TOTAL SOIL-LOSS VOLUME (ACRE-FEET) = 145.4045  
TOTAL STORM RUNOFF VOLUME (ACRE-FEET) = 345.3988  
-----

=====

2 4 - H O U R S T O R M  
R U N O F F H Y D R O G R A P H

=====

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)

(Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	250.0	500.0	750.0	1000.0
16.000	192.7135	335.17	.	.	Q	.	V
16.083	195.5213	407.69	.	.	Q	.	V
16.167	199.0795	516.65	.	.	Q	V	.
16.250	203.6414	662.40	.	.	.	V	Q
16.333	209.1870	805.22	.	.	.	V	Q
16.417	215.2608	881.91	.	.	.	V	Q
16.500	221.6831	932.53	.	.	.	V	Q
16.583	227.7637	882.91	.	.	.	V	Q
16.667	233.0724	770.83	.	.	.	V	Q

16.750	237.5088	644.16	.	.	.	Q	V	.
16.833	241.3148	552.63	.	.	.	Q	V	.
16.917	244.6089	478.30	.	.	.	Q	V	.
17.000	247.5778	431.09	.	.	.	Q	V	.
17.083	250.2464	387.48	.	.	.	Q	V	.
17.167	252.6820	353.64	.	.	.	Q	V	.
17.250	254.8753	318.48	.	.	.	Q	V	.
17.333	256.9194	296.80	.	.	.	Q	V	.
17.417	258.8325	277.78	.	.	.	Q	V	.
17.500	260.6190	259.40	.	.	.	Q	V	.
17.583	262.2833	241.65	.	.	.	Q	V	.
17.667	263.8473	227.09	.	.	.	Q	V	.
17.750	265.2838	208.59	.	.	.	Q	V	.
17.833	266.6406	197.00	.	.	.	Q	V	.
17.917	267.9378	188.36	.	.	.	Q	V	.
18.000	269.1791	180.23	.	.	.	Q	V	.
18.083	270.3817	174.62	.	.	.	Q	V	.
18.167	271.5547	170.32	.	.	.	Q	V	.
18.250	272.7091	167.62	.	.	.	Q	V	.
18.333	273.8674	168.19	.	.	.	Q	V	.
18.417	275.0389	170.10	.	.	.	Q	V	.
18.500	276.2276	172.60	.	.	.	Q	V	.
18.583	277.4275	174.22	.	.	.	Q	V	.
18.667	278.6409	176.18	.	.	.	Q	V	.
18.750	279.8600	177.01	.	.	.	Q	V	.
18.833	281.0784	176.92	.	.	.	Q	V	.
18.917	282.2896	175.86	.	.	.	Q	V	.
19.000	283.4850	173.58	.	.	.	Q	V	.

-----  
TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
(Note: 100% of Peak Flow Rate estimate assumed to have  
an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1145.0
10%	1145.0
20%	215.0
30%	125.0
40%	65.0
50%	50.0
60%	35.0
70%	30.0
80%	25.0
90%	15.0

\*\*\*\*\*

FLOW PROCESS FROM NODE 352.00 TO NODE 352.00 IS CODE = 11

>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<<

STREAM HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)

(Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	1000.0	2000.0	3000.0	4000.0
------------	-------------	---------	----	--------	--------	--------	--------

Time (min)	Inflow (CFS)	Outflow (CFS)	Q	V	Q	V
16.000	893.2413	1472.17	.	.	Q	. V
16.083	905.1295	1726.17	.	.	.	Q . V
16.167	920.0520	2166.74	.	.	.	.QV
16.250	938.9121	2738.49	.	.	.	. V Q
16.333	961.9019	3338.12	.	.	.	. V . Q
16.417	987.7637	3755.13	.	.	.	. V . Q
16.500	1014.9455	3946.80	.	.	.	. V . Q.
16.583	1040.9474	3775.47	.	.	.	. V . Q
16.667	1064.0802	3358.88	.	.	.	. V . Q
16.750	1083.9247	2881.42	.	.	.	. VQ .
16.833	1101.1487	2500.93	.	.	.	. Q V .
16.917	1116.3706	2210.22	.	.	.	. Q V .
17.000	1130.1700	2003.67	.	.	.	. Q V .
17.083	1142.8180	1836.48	.	.	.	. Q . V .
17.167	1154.4935	1695.29	.	.	.	. Q . V .
17.250	1165.2758	1565.59	.	.	.	. Q . V .
17.333	1175.3296	1459.82	.	.	.	. Q . V .
17.417	1184.7463	1367.32	.	.	.	. Q . V .
17.500	1193.5865	1283.59	.	.	.	. Q . V .
17.583	1201.8761	1203.65	.	.	.	. Q . V .
17.667	1209.6827	1133.53	.	.	.	. Q . V .
17.750	1217.0486	1069.52	.	.	.	. Q . V .
17.833	1224.0178	1011.94	.	.	.	. Q . V .
17.917	1230.6729	966.31	.	.	.	. Q . V .
18.000	1237.0647	928.09	.	.	.	. Q . V .
18.083	1243.2318	895.47	.	.	.	. Q . V .
18.167	1249.2446	873.06	.	.	.	. Q . V .
18.250	1255.1493	857.35	.	.	.	. Q . V .
18.333	1260.9901	848.09	.	.	.	. Q . V .
18.417	1266.8301	847.96	.	.	.	. Q . V .
18.500	1272.7109	853.90	.	.	.	. Q . V .
18.583	1278.6105	856.61	.	.	.	. Q . V .
18.667	1284.5020	855.45	.	.	.	. Q . V .
18.750	1290.3645	851.24	.	.	.	. Q . V .
18.833	1296.1793	844.31	.	.	.	. Q . V .
18.917	1301.9497	837.87	.	.	.	. Q . V .
19.000	1307.6602	829.16	.	.	.	. Q . V .

TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
 (Note: 100% of Peak Flow Rate estimate assumed to have  
 an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1145.0
10%	1145.0
20%	330.0
30%	160.0
40%	70.0
50%	55.0
60%	40.0
70%	30.0
80%	25.0
90%	15.0

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 352.00 TO NODE 353.00 IS CODE = 5.2  
 -----  
 >>>>MODEL CHANNEL ROUTING OF STREAM #1 BY THE CONVEX METHOD<<<<<  
 =====

THE MODIFIED C-ROUTING COEFFICIENT IS ESTIMATED IN ORDER  
 TO ROUTE THE STREAM 1 INFLOW HYDROGRAPH BY 5-MINUTE  
 INTERVALS(Reference: the National Engineering Handbook,  
 Hydrology, Chapter 17, page 17-52, August,1972,  
 U.S. Department of Commerce).

ASSUMED REGULAR CHANNEL INFORMATION:  
 BASEWIDTH(FT) = 12.00 CHANNEL Z = 2.00  
 UPSTREAM ELEVATION(FT) = 1655.00  
 DOWNSTREAM ELEVATION(FT) = 1625.00  
 CHANNEL LENGTH(FT) = 1454.79 MANNING'S FACTOR = 0.015  
 CONSTANT LOSS RATE(CFS) = 0.00

CHANNEL ROUTING COEFFICIENT ESTIMATED:  
 MAXIMUM INFLOW(CFS) = 3946.80  
 AVERAGE FLOWRATE IN EXCESS OF 50% MAXIMUM INFLOW = 2970.53  
 CHANNEL NORMAL VELOCITY FOR Q = 2970.53 CFS = 29.74 FPS  
 ESTIMATED CHANNEL ROUTING COEFFICIENT = 0.946

MODIFIED CHANNEL ROUTING COEFFICIENT FOR 5-MINUTE  
 UNIT INTERVALS IS CSTAR = 1.000

CONVEX METHOD CHANNEL ROUTING RESULTS:

MODEL TIME (HRS)	INFLOW (STREAM 1) (CFS)	ROUTED FLOW (CFS)	OUTFLOW LESS
			LOSS (STREAM 1) (CFS)
16.000	1472.17	1460.38	1460.38
16.083	1726.17	1687.00	1687.00
16.167	2166.74	2098.80	2098.80
16.250	2738.49	2650.31	2650.31
16.333	3338.12	3245.65	3245.65
16.417	3755.13	3690.82	3690.82
16.500	3946.80	3917.24	3917.24
16.583	3775.47	3801.90	3801.90
16.667	3358.88	3423.13	3423.13
16.750	2881.42	2955.05	2955.05
16.833	2500.93	2559.61	2559.61
16.917	2210.22	2255.05	2255.05
17.000	2003.67	2035.53	2035.53
17.083	1836.48	1862.27	1862.27
17.167	1695.29	1717.06	1717.06
17.250	1565.59	1585.59	1585.59
17.333	1459.82	1476.13	1476.13
17.417	1367.32	1381.59	1381.59
17.500	1283.59	1296.50	1296.50
17.583	1203.65	1215.98	1215.98
17.667	1133.53	1144.34	1144.34
17.750	1069.52	1079.39	1079.39

17.833	1011.94	1020.82	1020.82
17.917	966.31	973.34	973.34
18.000	928.09	933.99	933.99
18.083	895.47	900.50	900.50
18.167	873.06	876.52	876.52
18.250	857.35	859.77	859.77
18.333	848.09	849.51	849.51
18.417	847.96	847.98	847.98
18.500	853.90	852.99	852.99
18.583	856.61	856.19	856.19
18.667	855.45	855.63	855.63
18.750	851.24	851.89	851.89
18.833	844.31	845.38	845.38
18.917	837.87	838.86	838.86
19.000	829.16	830.50	830.50

PROCESS SUMMARY OF STORAGE:

INFLOW VOLUME = 1600.928 AF  
 OUTFLOW VOLUME = 1600.928 AF  
 LOSS VOLUME = 0.000 AF

\*\*\*\*\*

FLOW PROCESS FROM NODE 353.00 TO NODE 353.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<

(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERSHED AREA = 50.170 ACRES  
 BASEFLOW = 0.000 CFS/SQUARE-MILE  
 \*USER ENTERED "LAG" TIME = 0.100 HOURS  
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.  
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)  
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.  
 VALLEY (DEVELOPED):  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.433  
 FOOTHILL "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 MOUNTAIN "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 VALLEY (UNDEVELOPED) / DESERT:  
 "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.567  
 DESERT (UNDEVELOPED) "S"-CURVE PERCENTAGE (DECIMAL NOTATION) = 0.000  
 MAXIMUM WATERSHED LOSS RATE (INCH/HOUR) = 0.322  
 LOW LOSS FRACTION = 0.306  
 \*HYDROGRAPH MODEL #1 SPECIFIED\*

SPECIFIED PEAK 5-MINUTES RAINFALL (INCH) = 0.45  
 SPECIFIED PEAK 30-MINUTES RAINFALL (INCH) = 0.92  
 SPECIFIED PEAK 1-HOUR RAINFALL (INCH) = 1.22  
 SPECIFIED PEAK 3-HOUR RAINFALL (INCH) = 2.04  
 SPECIFIED PEAK 6-HOUR RAINFALL (INCH) = 2.83  
 SPECIFIED PEAK 24-HOUR RAINFALL (INCH) = 6.97

\*USER SPECIFIED PRECIPITATION DEPTH-AREA REDUCTION FACTORS:  
 5-MINUTE FACTOR = 0.820  
 30-MINUTE FACTOR = 0.820  
 1-HOUR FACTOR = 0.820

3-HOUR FACTOR = 0.973  
 6-HOUR FACTOR = 0.987  
 24-HOUR FACTOR = 0.992

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES  
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 83.333

RUNOFF HYDROGRAPH LISTING LIMITS:  
 MODEL TIME (HOURS) FOR BEGINNING OF RESULTS = 16.00  
 MODEL TIME (HOURS) FOR END OF RESULTS = 19.00

UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES (CFS)
1	13.170	79.908
2	62.562	299.685
3	85.799	140.986
4	92.075	38.078
5	94.971	17.573
6	96.847	11.385
7	98.101	7.603
8	98.891	4.797
9	99.262	2.251
10	99.597	2.031
11	99.839	1.468
12	99.960	0.734
13	100.000	0.245

TOTAL SOIL-LOSS VOLUME (ACRE-FEET) = 8.4464  
 TOTAL STORM RUNOFF VOLUME (ACRE-FEET) = 20.4475

2 4 - H O U R S T O R M  
 R U N O F F H Y D R O G R A P H

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)  
 (Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	50.0	100.0	150.0	200.0
16.000	12.3360	33.69	.	Q	.	V	.
16.083	12.7910	66.07	.	.	Q	.	V
16.167	13.6395	123.20	.	.	.	Q	V
16.250	14.1286	71.02	.	.	Q	.	V
16.333	14.3704	35.12	.	Q	.	.	V
16.417	14.5459	25.48	.	Q	.	.	V

16.500	14.7135	24.34	. Q	.	.	V .	.
16.583	14.8705	22.80	. Q	.	.	V.	.
16.667	15.0135	20.77	. Q	.	.	V.	.
16.750	15.1441	18.96	. Q	.	.	V.	.
16.833	15.2679	17.97	. Q	.	.	V.	.
16.917	15.3850	17.01	. Q	.	.	V	.
17.000	15.4958	16.10	. Q	.	.	V	.
17.083	15.5987	14.94	. Q	.	.	V	.
17.167	15.6871	12.83	. Q	.	.	V	.
17.250	15.7673	11.65	. Q	.	.	V	.
17.333	15.8433	11.03	. Q	.	.	V	.
17.417	15.9159	10.55	. Q	.	.	V	.
17.500	15.9858	10.15	. Q	.	.	V	.
17.583	16.0532	9.79	.Q	.	.	V	.
17.667	16.1185	9.48	.Q	.	.	V	.
17.750	16.1819	9.20	.Q	.	.	V	.
17.833	16.2435	8.95	.Q	.	.	V	.
17.917	16.3035	8.71	.Q	.	.	V	.
18.000	16.3621	8.50	.Q	.	.	V	.
18.083	16.4217	8.65	.Q	.	.	V	.
18.167	16.4890	9.78	.Q	.	.	V	.
18.250	16.5595	10.23	. Q	.	.	V	.
18.333	16.6301	10.25	. Q	.	.	V	.
18.417	16.7003	10.19	. Q	.	.	V	.
18.500	16.7699	10.10	. Q	.	.	V	.
18.583	16.8388	10.01	. Q	.	.	V	.
18.667	16.9071	9.91	.Q	.	.	V	.
18.750	16.9746	9.81	.Q	.	.	V	.
18.833	17.0415	9.71	.Q	.	.	V	.
18.917	17.1077	9.61	.Q	.	.	V	.
19.000	17.1732	9.51	.Q	.	.	V	.

-----  
TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
(Note: 100% of Peak Flow Rate estimate assumed to have  
an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
=====	=====
0%	1145.0
10%	185.0
20%	35.0
30%	15.0
40%	15.0
50%	15.0
60%	5.0
70%	5.0
80%	5.0
90%	5.0

\*\*\*\*\*  
FLOW PROCESS FROM NODE 353.00 TO NODE 353.00 IS CODE = 11

-----  
>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<<  
=====

STREAM HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS (CFS)

(Note: Time indicated is at END of Each Unit Intervals)

TIME (HRS)	VOLUME (AF)	Q (CFS)	0.	1000.0	2000.0	3000.0	4000.0
16.000	904.0137	1494.07	.	.	Q	. V	.
16.083	916.0872	1753.06	.	.	Q	. V	.
16.167	931.3902	2222.00	.	.	.	Q	.
16.250	950.1321	2721.33	.	.	. V	Q	.
16.333	972.7269	3280.77	.	.	.	V	. Q
16.417	998.3213	3716.30	.	.	.	V	. Q
16.500	1025.4672	3941.58	.	.	.	V	. Q
16.583	1051.8080	3824.69	.	.	.	V	. Q
16.667	1075.5262	3443.90	.	.	.	V	. Q
16.750	1096.0084	2974.01	.	.	.	V	Q.
16.833	1113.7604	2577.58	.	.	.	Q	V
16.917	1129.4082	2272.06	.	.	.	Q	V
17.000	1143.5378	2051.62	.	.	.	Q	V
17.083	1156.4662	1877.20	.	.	.	Q	V
17.167	1168.3800	1729.89	.	.	.	Q	V
17.250	1179.3802	1597.24	.	.	.	Q	V
17.333	1189.6224	1487.16	.	.	.	Q	V
17.417	1199.2102	1392.14	.	.	.	Q	V
17.500	1208.2092	1306.65	.	.	.	Q	V
17.583	1216.6511	1225.77	.	.	.	Q	V
17.667	1224.5975	1153.82	.	.	.	Q	V
17.750	1232.0947	1088.59	.	.	.	Q	V
17.833	1239.1868	1029.77	.	.	.	Q	V
17.917	1245.9502	982.06	.	.	.	Q	V
18.000	1252.4412	942.49	.	.	.	Q	V
18.083	1258.7025	909.15	.	.	.	Q	V
18.167	1264.8065	886.30	.	.	.	Q	V
18.250	1270.7983	870.01	.	.	.	Q	V
18.333	1276.7196	859.77	.	.	.	Q	V
18.417	1282.6299	858.17	.	.	.	Q	V
18.500	1288.5741	863.09	.	.	.	Q	V
18.583	1294.5397	866.21	.	.	.	Q	V
18.667	1300.5007	865.54	.	.	.	Q	V
18.750	1306.4353	861.70	.	.	.	Q	V
18.833	1312.3243	855.09	.	.	.	Q	V
18.917	1318.1678	848.47	.	.	.	Q	V
19.000	1323.9530	840.01	.	.	.	Q	V

-----  
TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:  
(Note: 100% of Peak Flow Rate estimate assumed to have  
an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
=====	=====
0%	1145.0
10%	1145.0
20%	355.0
30%	160.0
40%	75.0
50%	55.0
60%	40.0
70%	30.0
80%	25.0

=====

END OF FLOODSCx ROUTING ANALYSIS

1